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APPLICATION OF COMBINED JOINTED MEDIA
AND DISCRETE SLIP PLANE CHARACTERISTICS
TO SUBSIDENCE PREDICTIONS

DAVID W. BASINGER

APPLICATION OF COMBINED JOINTED MEDIA
AND DISCRETE SLIP PLANE CHARACTERISTICS
TO SUBSIDENCE PREDICTIONS

A Thesis

Presented to the
Department of Civil Engineering
Brigham Young University

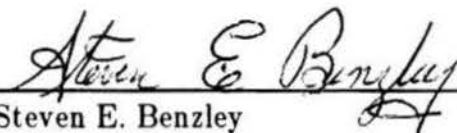
In Partial Fulfillment
of the Requirements for the Degree
Master of Science

by

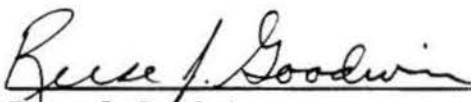
David W. Basinger

December 1984

This Thesis, by David W. Basinger, is accepted in its present form by the Department of Civil Engineering of Brigham Young University as satisfying the thesis requirement for the degree of Master of Science.

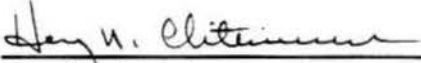


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CHAPTER ONE

INTRODUCTION

Ground settlement, also called subsidence, has increased in significance as the use of longwall mining and underground coal gasification have become more prominent. Many methods have been used to predict the amount of subsidence that may occur. These include empirical methods, physical models, and numerical models. This thesis investigates the use of finite element material modeling to incorporate slippage planes in the analysis of subsidence problems.

Chapter two of this thesis reviews current research in finite-element numerical simulations of subsidence predictions. The area emphasized is the ability to incorporate fractures and joints in rock mechanics problems.

In chapter three, the theory behind two methods of modeling fractures in finite element technique is presented. Low shear theory is presented as the first method. Among the items discussed in low shear theory are: (1) the incorporation of low shear slip planes into the model, (2) the formulation of the low shear, and (3) the installation of a low shear element row into the model. The second method used to modeling fractures is to model the material as a jointed media continuum. In this section, the topics are: (1) the approximation of the media as a continuum, (2) the formulation of a constitutive model, (3) the assumptions of model behavior,

and (4) the solution of the constitutive equations. Three basic assumptions are included in the jointed media formulation. These involve: (1) the behavior of intact material, (2) the neglect of joint dilatation, and (3) the behavior of joint shear.

In chapter four, various models used to predict subsidence in a coal mine problem are shown. First, a physical centrifuge model carried out at Sandia National Laboratories is described in detail. The results of this model are also given. Second, several numerical models set up and analyzed using the SCRUBS.BYU finite element program are described. Included in the descriptions are: (1) the equipment used, (2) the construction of the model, (3) the dimensions of the model, (4) the materials used, (5) the creation of the subsurface cavity, and (6) the results of selected runs. Finally, comparisons are made between the models.

In chapter five, conclusions are made and recommendations listed. The successes, the problems, and the proposed solution to these problems are presented.

In the attached appendices, a current listing of the program SCRUBS.BYU is provided. Also included are a subroutine map, a short description of each subroutine, additional results of simulation runs, and sample files used in the simulations.

CHAPTER TWO

SUBSIDENCE PREDICTION

Background Information

The collapse of the earth at the surface, called surface subsidence, can seriously effect ground water, surface hydrology, and surface structures. The mining of natural resources, especially of coal, can result in significant amounts of subsidence that in turn can cause significant damage.

Consequently, accurate subsidence prediction is of great interest to mining engineers. Recently, increased use of longwall mining techniques and increased research in underground coal gasification have heightened the concern in predicting the behavior of rocks and soils subject to mining stresses [17]. Longwall mining and in-situ (underground) coal gasification are often preferred as methods that maximize resource removal, as both of these methods have the result that all the coal in a seam is removed. The result is, of course, the total collapse of the area above the mined region and the resulting subsidence of all overlying strata. This is in contrast to the older mining method of room and pillar mining, where pillars of coal were left behind to support the mine roof.

Methods of Prediction

Historical methods. Empirical methods have historically been used to predict surface subsidence [3,14]. These methods are based mostly

on engineering judgement and require a large data base from which to derive the design formulas. In Europe, where empirical methods have long been used, many records exist that provide a long history from which to gather a sufficient data base. In the United States such a data base does not usually exist and alternative methods are sought. Methods that have been investigated include both physical and numerical models. It has been shown that mutual enhancement can occur when both physical and numerical models are applied to subsidence problems [19]. In this thesis, physical and numerical methods are applied to a subsidence problem in an effort to obtain the additional understanding that this mutual enhancement can afford.

Physical methods. A centrifuge can be used to physically simulate mining subsidence. Subsidence above a mine is caused primarily by the force of gravity. Gravity interacting with the mine geometry, the material properties of the rock and soil, and the natural ground movement of the area, creates a stress field in the overburden. For an accurate model, the stress field in the full-size structure must be duplicated in the reduced-scale model. To do that, the force of gravity can be increased in the same proportion that the model has been reduced in scale. A centrifuge can be used to apply these greater-than-gravity loads to the scaled model. For example, a coal mine at a depth of 100 m below ground level can be simulated by a scale model only 1 m high and constructed from representative site materials by loading the scaled model in a centrifuge to 100 times the acceleration of gravity (g 's).

Numerical methods. Numerical models have increased in use with the onset of the computer, and the corresponding increase in computational power, speed, and accuracy. Many numerical approaches to subsidence prediction have been used [1,7,13,22,23]. One important numerical technique is called the finite element technique.

The finite element technique is a numerical method that has been adapted to modeling geologic materials. The nonhomogeneity of soil and rocks make their modeling intrinsically complicated and uncertain. Material properties are often vague. The presence of cracks and fissures increase the complexity and difficulty of predicting the behavior of geologic materials. As a result, numerical simulations of geologic materials have almost necessarily involved major idealizations and simplifications involving the material properties and the effect of fractures and joints on subsidence mechanics.

While accurately determining soil properties will always be a problem, determining the location of cracks and joints and incorporating their effects into a numerical model leads to better and more accurate predictions of soil and rock mechanics problems. Many finite element codes have incorporated joints and fractures into their analysis by adding sliding interfaces or slip planes, with the usual result that computation time has increased dramatically [16]. At Brigham Young University, several unique geologic modeling techniques have been included in a finite element program called SCRUBS.BYU.

Current Research

The computer code SCRUBS.BYU. SCRUBS.BYU is a two-dimensional, elastic-plastic finite element computer program developed to analyze resource extraction problems resulting in significant surface subsidence [9]. SCRUBS.BYU can model the subsidence process associated with either longwall coal mining or in-situ coal gasification. Of special interest in SCRUBS.BYU'S subsidence formulations are the abilities to model failure and rubbleization, to mine elements, and to incorporate joints and cracks.

Fracture modeling. In 1983 Royd Nelson, a master's candidate at Brigham Young University, incorporated joints, cracks, and fractures into SCRUBS.BYU [16]. Two separate crack definitions were used. In the first, a discrete crack model was used, and in the other a continuum approach defining a new material with regularly spaced joints was used. These capabilities were demonstrated by several simplified examples.

Recommendations for future study. Further study in this area could be made. Nelson specifically recommended that areas of future work include [16, p.56]:

- 1) Studying the effects of faults on a variety of geomechanical problems using the slip plane addition to model the faults.
- 2) Evaluating the effect of joints and fractures on problems where the rubble model available in SCRUBS.BYU is utilized.
- 3) Investigating plastic yielding of the low shear slip plane material, particularly with utilization of the Drucker-Prager failure criteria.

- 4) Performing experimental studies to determine single-joint slip responses in order to more accurately estimate the material constants that define the slip response.
- 5) Performing of large-scale tests to verify analytical calculations.

To some extent this thesis presents further study in each of the recommended areas except number 3. The investigation of plastic yielding and the Drucker-Prager failure criteria was deemed to be beyond the scope of this study.

The Scope of this thesis. This thesis is a continuation of Nelson's work to incorporate fracture modeling into the finite element code SCRUBS.BYU. Verification of the discrete slip plane and jointed media formulations is sought by their application to a physical model. In addition, the effort to combine discrete slip planes and jointed media continua in models will increase the understanding of numerical geologic materials modeling.

CHAPTER THREE

LOW SHEAR AND JOINTED MEDIA THEORY

Low Shear Theory

Modeling with low shear element slip planes. The modeling of joints and fractures has been incorporated into many finite element codes by using sliding interfaces or slip planes. The implementation of these interfaces into the codes involves complex formulations and often increases the computing time of these programs considerably. A slip plane formulation is part of the finite element computer code SCRUBS.BYU. In this program a "low shear element" slip plane was formulated. The low shear element slip plane has the advantages of being easy to implement and has a limited effect on the run time of the program. In this method, a thin row of elements is added into the model wherever a slip plane is desired, as shown in Figure 1. This thin row of elements is then defined as a low shear material. This formulation simply imposes regions of minimal shear resistance along the slip plane element row.

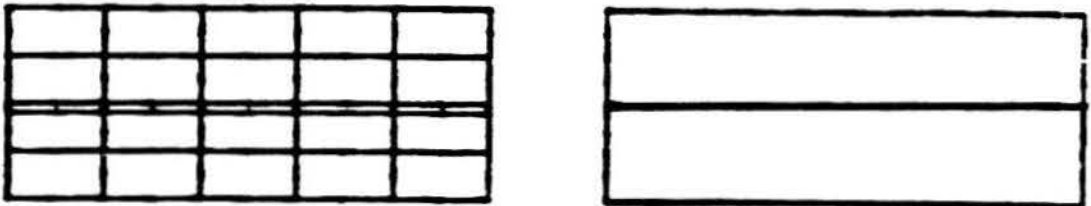


Figure 1. Low shear slip plane addition.

The low shear formulation.

"In standard plane strain computer programs the relationship between stresses and strains is expressed as a generalized form of Hooke's law,

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \frac{E}{(1+\nu)(1-2\nu)} \begin{bmatrix} (1-\nu) & \nu & 0 \\ \nu & (1-\nu) & 0 \\ 0 & 0 & \frac{(1-2\nu)}{2} \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \gamma_{xy} \end{Bmatrix}$$

Problems occur in this formulation. When Poisson's ratio becomes .5, division by zero occurs. Another disadvantage for a material that has little shear strength is that as the shear modulus G approaches zero, the modulus of Elasticity E also approaches zero and the equations for stress and strain break down.

In a manner similar to Clough and Woodward [4] we can circumvent the above problems by writing the stress strain relations in terms of the bulk modulus (K) and the shear modulus (G). Doing this,

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \begin{bmatrix} K + \frac{4}{3}G & K - \frac{2}{3}G & 0 \\ K - \frac{2}{3}G & K + \frac{4}{3}G & 0 \\ 0 & 0 & G \end{bmatrix} \begin{Bmatrix} \epsilon_{xx} \\ \epsilon_{yy} \\ \gamma_{xy} \end{Bmatrix}$$

where

$$K = \frac{E}{3(1-2\nu)}$$

$$G = \frac{E}{2(1+\nu)}$$

The above equation allows G to approach zero and still maintain a valid stress strain relationship." [2]

This formulation is incorporated in the SCRUBS.BYU program by entering the bulk modulus K as a negative number in place of Young's modulus, and by entering the shear modulus G in place of Poisson's ratio. The negative sign on the bulk modulus is a flag that causes the elasticity

matrix to be created using equation 3.2 rather than equation 3.1. To model a low shear material, the shear modulus is entered as a number low relative to the bulk modulus. As stated before, this formulation is not limited to use in low shear materials, but can be utilized any time the material is described by K and G rather than by E and ν .

PRESCRUBS.BYU provides the capability to input low shear slip planes. A solution control parameter is required that indicates whether any slip planes exist. If so, the user is automatically prompted for the bulk and shear moduli for the slip plane material. Note that this formulation requires a separate boundary flag for each discrete slip plane.

Addition of the slip plane element row. The geometric modeling is done on an interactive mesh generator called QMESH.BYU. This program accepts the perimeter data for specified regions of the geometry, and automatically creates and numbers the finite element mesh. Boundary conditions are imposed by assigning flags to desired lines or line segments that can later (in PRESCRUBS.BYU) be coupled to specified displacement or force criteria. To define low shear element rows as described earlier, the user would have to enter perimeter data for each row. The SCRUBS.BYU program automatically creates this thin row of elements. Any line can be defined as a slip plane by simply assigning it a boundary flag in QMESH.BYU, thus saving considerable preprocessing time. (Readers not familiar with the use of QMESH.BYU should consult Reference 8.)

Jointed Media Theory

Modeling as jointed media. A second method of modeling the effects of cracks and fractures in geologic materials is by using a material model for the jointed material based on continuum theory. In this method, joints in the materials are implemented by allowing for the effects of jointing on the material response rather than discrete models for each joint. This formulation is taken from work done by Thomas [21], but is modified neglecting joint dilatation response, and limited to only two dimensional or axisymmetric continua.

The model is composed of two parts: (1) a continuum approximation in which the joint displacements are averaged through the material, and (2) a continuum description based on linear behavior of the base material and nonlinear shear at the joints.

The continuum approximation. The continuum model used for this material is based on the published work of Moreland [10,11,12]. Consider a "representative elementary volume" containing regularly spaced parallel fractures as sketched in Figure 2. The orientation of this joint set is characterized by a unit normal vector \hat{n} with respect to fixed x_1, x_2, x_3 coordinate axes. The spacing between fractures is denoted by δ . Additional unit vectors \hat{s} and \hat{t} in the plane of the joints are introduced such that \hat{s} , \hat{t} , and \hat{n} form a local Cartesian coordinate system.

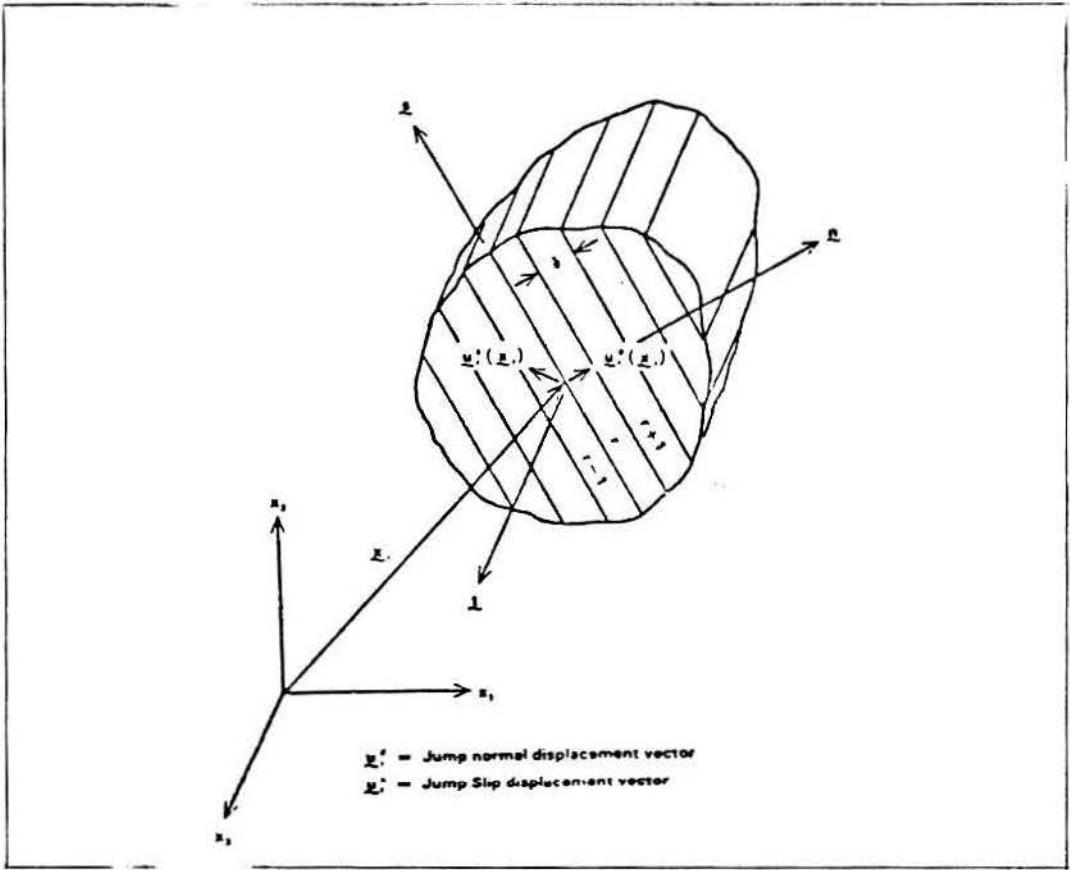


Figure 2. Representative elementary volume containing regularly spaced fractures.

It is assumed that the relative motion at the interface of the r th fracture at position x_r can be measured by a jump "dilatation" vector $u_r^d(x_r)$ normal to the fracture plane, and a jump slip displacement vector $u_r^s(x_r)$ parallel to the fracture plane. The net jump displacements for R fractures in the representative volume will then be

$$R\bar{u}^d(\underline{x}) = \sum_{r=1}^R \underline{u}_r^d(\underline{x}_r)$$

$$R\bar{u}^s(\underline{x}) = \sum_{r=1}^R \underline{u}_r^s(\underline{x}_r) \quad (3.3)$$

where \bar{u}^d and \bar{u}^s are average displacements and \underline{x} is any position in the element. The continuous displacement fields \bar{u}^d and \bar{u}^s with respect to the x_1, x_2, x_3 axes are introduced,

$$\underline{u}^d(\underline{x} + d\underline{n}) = \underline{u}^d(\underline{x}) + \left[\frac{\bar{u}^d}{\delta} \right] d\underline{n}$$

$$\underline{u}^s(\underline{x} + d\underline{n}) = \underline{u}^s(\underline{x}) + \left[\frac{\bar{u}^s}{\delta} \right] d\underline{n}, \quad (3.4)$$

where

$$\bar{u}^d = |\bar{u}^d| \underline{n} = \bar{u}^d \underline{n}$$

$$\bar{u}^s = |\bar{u}^s| \underline{v} = \bar{u}^s \underline{v} \quad (3.5)$$

In equation (3.5) the direction of slip displacement is in the direction of the unit vector \underline{v} . The total displacements can be written as

$$\underline{u}(\underline{x}) = \underline{u}^b(\underline{x}) + \underline{u}^d(\underline{x}) + \underline{u}^s(\underline{x}) \quad (3.6)$$

where \underline{u}^b is the displacement field of the intact material between fractures.

From equation (3.6), the strain decomposition is taken to be

$$\underline{\underline{e}} = \underline{\underline{e}}^b + \underline{\underline{e}}^d + \underline{\underline{e}}^s \quad (3.7)$$

The dilatation and slip strains are defined in terms of the continuous displacement,

$$\begin{aligned} 2 \underline{\underline{e}}^d &= \underline{u}^d \nabla + (\underline{u}^d \nabla)^T \\ 2 \underline{\underline{e}}^s &= \underline{u}^s \nabla + (\underline{u}^s \nabla)^T \end{aligned} \quad (3.8)$$

where ∇ is the gradient operator with respect to the x_1, x_2, x_3 axes. Equation (3.8) can be reduced by decomposing \underline{u}^d and \underline{u}^s into components in the local coordinate system.

$$\begin{aligned} \underline{u}^d &= |\underline{u}^d| \underline{n} = u^d \underline{n} \\ \underline{u}^s &= |\underline{u}^s| \underline{v} = u_s^s \underline{s} \end{aligned} \quad (3.9)$$

Since both \underline{u}^d and \underline{u}^s have nonzero gradients only in the n direction, equation (3.8) becomes

$$\begin{aligned} \underline{\underline{e}}^d &= \frac{\partial u^d}{\partial n} (\underline{n} \times \underline{n}) \\ \underline{\underline{e}}^s &= \frac{1}{2} \frac{\partial u_s^s}{\partial n} (\underline{s} \times \underline{n} + \underline{n} \times \underline{s}) \end{aligned} \quad (3.10)$$

From equation (3.4)

$$\begin{aligned}\frac{\partial u^d}{\partial n} &= \frac{\bar{u}^d}{\delta} \\ \frac{\partial u^s}{\partial n} &= \frac{\bar{u}^s}{\delta}\end{aligned}\tag{3.11}$$

so the final form for the strains is

$$\begin{aligned}\underline{\underline{e}}^d &= \frac{\bar{u}^d}{\delta} (\underline{\underline{n}} \times \underline{\underline{n}}) \\ \underline{\underline{e}}^s &= \frac{\bar{u}^s}{2\delta} (\underline{\underline{s}} \times \underline{\underline{n}} + \underline{\underline{n}} \times \underline{\underline{s}})\end{aligned}\tag{3.12}$$

The constitutive model. We will first introduce the components of a stress tensor $\underline{\underline{T}}$ and a total strain tensor $\underline{\underline{E}}$ which refer to the local $\underline{\underline{s}}, \underline{\underline{t}}, \underline{\underline{n}}$ coordinate system. If σ and e are stress and strain tensors in the x_1, x_2, x_3 coordinate system, the transformation equations are

$$\begin{aligned}T_{nn} &= \underline{\underline{\sigma}}_{\underline{\underline{n}} \cdot \underline{\underline{n}}} \\ T_{ss} &= \underline{\underline{\sigma}}_{\underline{\underline{s}} \cdot \underline{\underline{s}}} \\ T_{tt} &= \underline{\underline{\sigma}}_{\underline{\underline{t}} \cdot \underline{\underline{t}}} \\ T_{ns} &= \underline{\underline{\sigma}}_{\underline{\underline{n}} \cdot \underline{\underline{s}}}\end{aligned}\tag{3.13}$$

and

$$\begin{aligned}
 E_{nn} &= \underline{\underline{\dot{\epsilon}}}^n \cdot \underline{\underline{\dot{\epsilon}}}^n \\
 E_{ss} &= \underline{\underline{\dot{\epsilon}}}^s \cdot \underline{\underline{\dot{\epsilon}}}^s \\
 E_{tt} &= \underline{\underline{\dot{\epsilon}}}^t \cdot \underline{\underline{\dot{\epsilon}}}^t \\
 E_{ns} &= \underline{\underline{\dot{\epsilon}}}^n \cdot \underline{\underline{\dot{\epsilon}}}^s
 \end{aligned} \tag{3.14}$$

The behavior of intact material. In the present formulation, the intact material is assumed to behave as a linear elastic solid, having a strain rate

$$\underline{\underline{\dot{\epsilon}}}^b = \frac{\dot{\sigma}}{2G} - \left(\frac{K - \frac{2}{3}G}{6KG} \right) (\text{tr} \underline{\underline{\dot{\sigma}}}) \underline{\underline{\dot{\epsilon}}}^I \tag{3.15}$$

Neglection of joint dilatation. The main purpose of using this model is to predict the weakening effects of slip along fractures and joints in materials. For many problems the fracture opening is small relative to the fracture spacing. For these problems, the joint dilatation normal to the crack is also small for cracks in compression; therefore the dilatation response will be neglected; thus

$$\underline{\underline{\dot{\epsilon}}}^d = 0 \tag{3.16}$$

The only condition imposed on the joint dilatation is that a joint cannot support a tensile load; therefore

$$T_{nn} = 0, \quad \bar{u}^d \geq 0 \tag{3.17}$$

The behavior assumption for joint shear. The joint shear stress-displacement behavior is assumed to be elastic perfectly plastic. In the elastic range

$$\dot{\bar{u}}_s^s = \frac{\dot{T}_{ns}}{G_s} \quad (3.18)$$

where G_s is an elastic modulus of slip determined for a single joint. From equation (3.12) the slip strain rate is

$$\dot{\bar{e}}^s = \frac{\dot{T}_{ns}}{2\delta G_s} (\bar{s} \times \bar{n} + \bar{n} \times \bar{s}) \quad (3.19)$$

The onset of plastic behavior is assumed to be governed by a linear Mohr-Coulomb criterion, based on a scalar "slip function", F , defined as

$$F = |T_{ns}| + \mu T_{nn} - C_o \quad (3.20)$$

where μ is the coefficient of friction and C_o is the cohesion. The joint behavior is elastic for F less than zero and plastic for F greater than zero. The stress-displacement behavior in shear is shown in Figure 3.

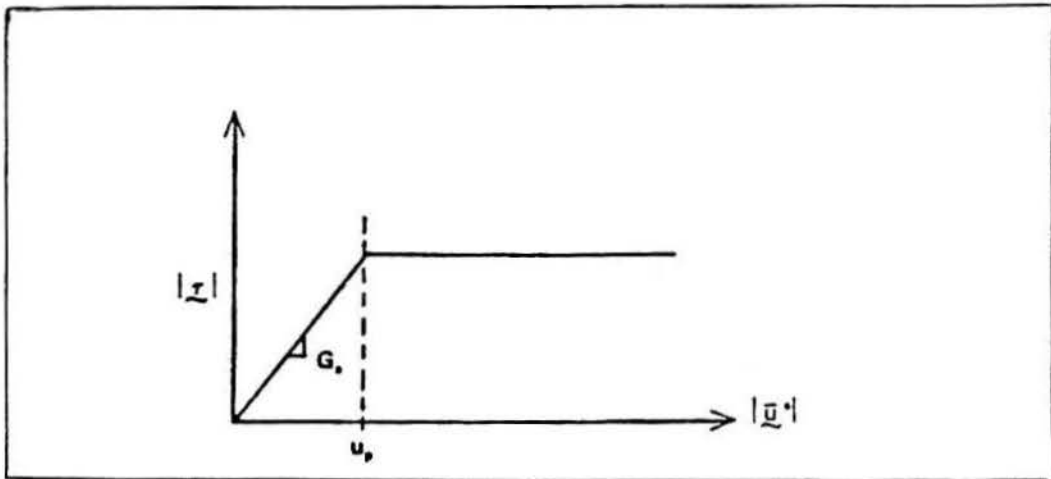


Figure 3. Assumed stress-displacement behavior in shear.

The solution of the constitutive equations. The SCRUBS.BYU finite element program uses an incremental method where the stresses and strains are known at the last stress increment, the current incremental strains are known, and only the current incremental stresses are needed.

From equation (3.7), the elastic strain rate can be defined as

$$\dot{\underline{\underline{\epsilon}}} = \dot{\underline{\underline{\epsilon}}^b} + \dot{\underline{\underline{\epsilon}}^d} + \dot{\underline{\underline{\epsilon}}^s}$$

Using the constitutive relations in equations (3.15), (3.16), and (3.19), the elastic strain rate [Eq 3.21] becomes

$$\dot{\underline{\underline{\epsilon}}} = \frac{\dot{\underline{\underline{\sigma}}}}{2G} - \left[\frac{K - \frac{2}{3}G}{6KG} \right] (\text{tr } \dot{\underline{\underline{\sigma}}}) \underline{\underline{I}} + \frac{\dot{T}_{ns}}{2\delta G_s} (\underline{\underline{s}} \times \underline{\underline{n}} + \underline{\underline{n}} \times \underline{\underline{s}})$$

Transferring equation (3.22) to scalar equations in terms of the local stress components, and omitting the out-of-plane shears, we have

$$\begin{aligned}
\dot{E}_{nn} &= \frac{\dot{T}_{nn}}{2G} - \left(\frac{K - \frac{2}{3}G}{6KG} \right) (\text{tr} \dot{T}) \\
\dot{E}_{ns} &= \left(\frac{1}{2G} + \frac{1}{2\delta G_s} \right) \dot{T}_{ns} \\
\dot{E}_{ss} &= \frac{\dot{T}_{ss}}{2G} - \left(\frac{K - \frac{2}{3}G}{6KG} \right) (\text{tr} \dot{T}) \\
\dot{E}_{tt} &= \frac{\dot{T}_{tt}}{2G} - \left(\frac{K - \frac{2}{3}G}{6KG} \right) (\text{tr} \dot{T}) \tag{3.23}
\end{aligned}$$

Solving for incremental stresses in terms of incremental strains produces

$$\begin{aligned}
\dot{T}_{ns} &= \left(\frac{2G}{1 + \frac{G}{\delta G_s}} \right) \dot{E}_{ns} \\
\dot{T}_{nn} &= 2G\dot{E}_{nn} + \left(K - \frac{2}{3}G \right) \text{tr} (\dot{E}) \\
\dot{T}_{ss} &= 2G\dot{E}_{ss} + \left(K - \frac{2}{3}G \right) \text{tr} (\dot{E}) \\
\dot{T}_{tt} &= 2G\dot{E}_{tt} + \left(K - \frac{2}{3}G \right) \text{tr} (\dot{E}) \tag{3.24}
\end{aligned}$$

Therefore for the elastic case, all components of incremental stress in the local coordinate system can be obtained directly from incremental strains. Next, it must be determined if plastic slip occurs. After the stress components have been obtained the slip function [Eq 3.20] must be tested.

If the slip function F was less than zero, the strain rate was entirely elastic and the stress rates determined in equation (3.24) are correct. However, if F was greater than zero, plastic slip has occurred.

For plastic slip, determination of the incremental stresses can easily be made by assuming elastic and perfectly plastic behavior such that

$$\dot{E}_{ns} = \dot{E}_{ns}^e + \dot{E}_{ns}^p \quad (3.25)$$

where \dot{E}_{ns} is the total strain increment, and \dot{E}_{ns}^e and \dot{E}_{ns}^p are the elastic and plastic components of incremental strain, and the shear stress increment is then defined as

$$\dot{T}_{ns} = \left(\frac{2G}{1 + \frac{G}{\delta G_s}} \right) E_{ns}^p \quad (3.26)$$

and the normal shear stresses are still defined as in equation (3.24) since they are not dependent on the incremental shear stress.

Using equations (3.25) and (3.26) and substituting into equation (3.20), the plastic strain, E_{ns}^p can be solved for directly by solving the slip function, F , equal to zero. After the plastic strain is determined the incremental shear stress can be determined by repeating equations (3.25) and (3.26).

In chapter four a physical centrifuge simulation performed previously at Sandia National Laboratories along with several simulations performed for this thesis are presented. The results of the numerical simulations are then compared with each other and with the results of the physical simulation.

CHAPTER FOUR

MODEL VERIFICATION

Description of Problem

This chapter summarizes the procedure and results of various mine subsidence simulations. First, a physical centrifuge experiment of a coal mine model done at Sandia National Laboratories and the results of that experiment are presented. Second, numerical simulations using SCRUBS.BYU are introduced, the variables of the numerical models are listed and the results shown. Finally, comparisons are made.

Centrifuge Model

The following description of the test was taken basically from the laboratory report of the test [20].

The prototype. This centrifuge experiment modeled the response of shale overburden to the mining of a long wall panel. The prototype mine was located 86.3 m below the surface. The primary material above this mine was Devonian shale with horizontal bedding planes. The shale was fractured along and across these bedding planes. Above the shale was a relatively thin layer of sandy soil. A cavity of increasing width beneath the shale was created and the resulting failures and rubble formation were recorded.

The Sandia 25-Foot Centrifuge was used to conduct this test. The scaled-down model described in this section was mounted on a "swing platform" connected to the centrifuge.

Construction of the model. To construct the model, several shale boulders were cut into 202 mm slabs using a large rock saw. Three of these slabs were chosen. Their lengths were trimmed to 1.167 m (or less) and their top and bottom surfaces were machined flat and parallel to their bedding planes. They were then placed in a holding fixture that duplicated the internal dimensions of the test fixture. After this initial processing, dental plaster was used to fill voids between the three slabs of shale and to fill voids around the ends of each slab.

The model was fractured to simulate a jointed shale overburden. The fracturing process involved machining grooves into the exposed surface of the model (along the bedding planes). The grooves were 6 mm wide by 13 mm deep and on 25 mm centers. A 1.167 m long knife blade was then pressed into each groove to crack the shale along the bedding planes (the plaster was also broken). Some vertical cracks were also generated in the model during handling, machining, and fracturing processes. A complete description of the crack system, as observed from the non-grooved face of the model, is given in Figure 4.

Dimensions of the model. After the model was fractured, a thin layer of silica sand was placed above the shale to represent the soil layer. The thickness of this layer was 63.5 mm. With the shale layers being 0.511 m high, the completed model was rectangular, 1.167 m long, 0.202 m deep and 0.575 m high (again, refer to Figure 4).

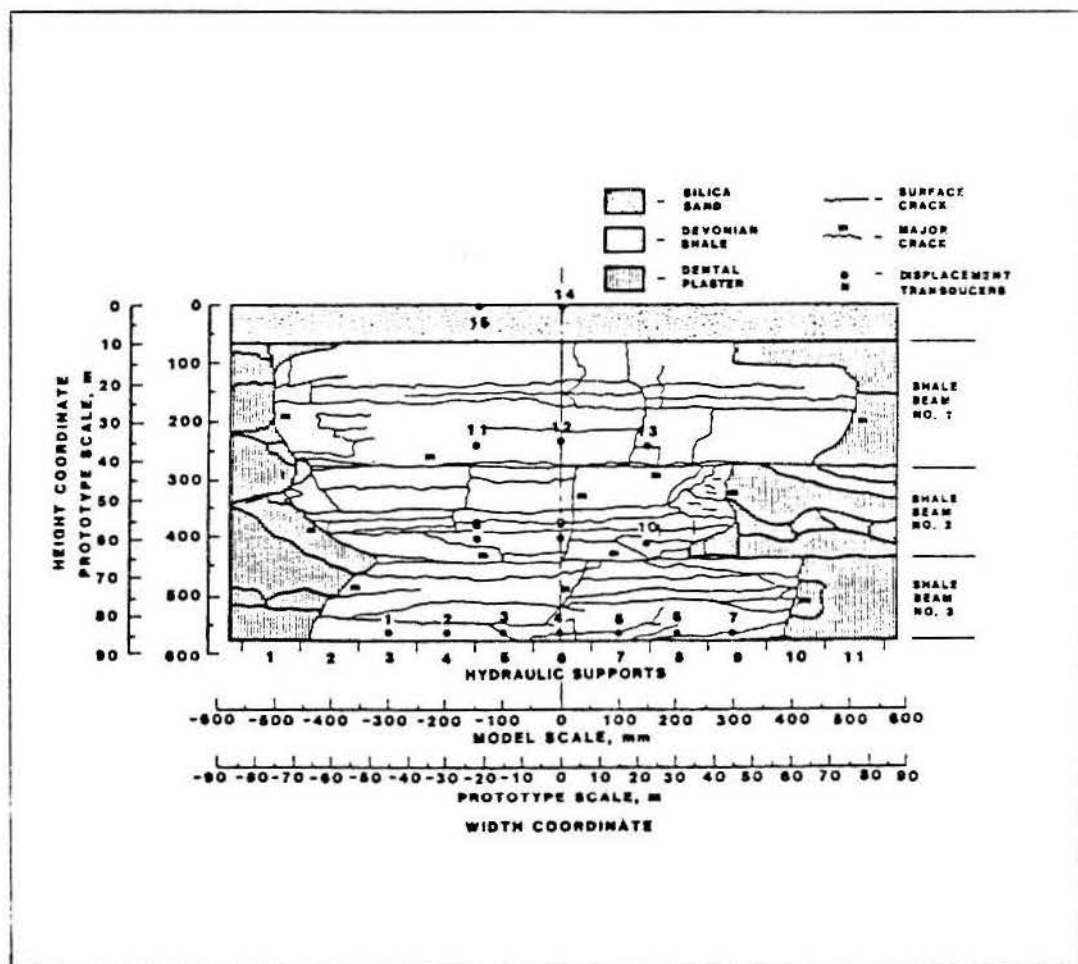


Figure 4. Schematic diagram of the centrifuge test [19].

The materials. As mentioned, three materials were used to construct the model mine structure. They were Devonian shale, silica sand, and dental plaster. The specific gravity of the Devonian shale was 2.64 Mg/m³. Its material properties varied between samples and with load range. For samples tested below 34.5 MPa, the average Modulus of Elasticity was 69.0 Gpa and Poisson's Ratio was 0.29. The unconfined compressive strength was 216 Mpa. The density of the dental plaster was

1.65 Mg/m³. The silica sand had a maximum particle size of 400 micrometers. When compacted at 150 g's, the density of this material was 1.25 Mg/m³.

Modeling the cavity. To create the subsurface cavity, the model was placed into the fixture with its bottom surface (length dimension) resting on a series of hydraulic supports. Each unit supported an area 102 mm wide by 203 mm deep. A total of 11 units were used to support the entire width (1.167 m) of the model. Using an electrically controlled hydraulic system, each support could be made to fall while under gravity loads. The cavity created by the fall of each support was 102 mm wide, 51 mm high, and 203 mm deep. Figure 4 includes a schematic diagram of this experimental configuration.

Between each segment of the simulation experiment (i.e., increase in width of the mine cavity), the centrifuge was stopped and additional records were made.

The experiment was conducted at an acceleration of 150 g's. Since linear dimensions scale in direct proportion with the acceleration, this model simulates a mine that is 86.3 m below the surface. The release of any single hydraulic support creates a prototype scale void that is 15.24 m wide and 7.62 m high. All subsequent presentation of dimensions in this section will be specified in the prototype scale.

The results. Before the cavity was created in the experiment, the model was taken to 150 g's for approximately 5 minutes. This period was used to stabilize it. This was considered to be the first "time step."

As discussed earlier, a total of 11 hydraulic units supported the model. The mine cavity was created by lowering chosen supports in four additional time steps. For this simulation, the chosen sequence for lowering the supports was symmetric about the centerline of the model. First, the centerline unit was dropped. Subsequently, symmetric pairs about the centerline were dropped. Using the numbering sequence shown in Figure 4, support number 6 was dropped first (time step two), next supports 5 and 7 were dropped simultaneously (time step three), then supports 4 and 8 (time step four), and, finally, supports 3 and 9 (time step five).

Dropping support 6 produced a cavity of 15.24 m wide by 7.62 m high below the total strata depth of 84.3 m.

When the width of the drift was increased to 45.7 m by dropping jacks 5 and 7 (a total of three supports removed), a failure arch was formed. This arch extended to a depth of 49.1 m below the surface (a height of 35.2 m above the original roof of the cavity).

When the width was increased in the next step to 76.2 m (a total of 5 supports removed), the failure extended to the surface. Some new cracks were formed. The surface subsidence trough that was introduced into the model by these motions had a lateral extent of 94 m with a maximum subsidence of 5.33 m (see Figure 5).

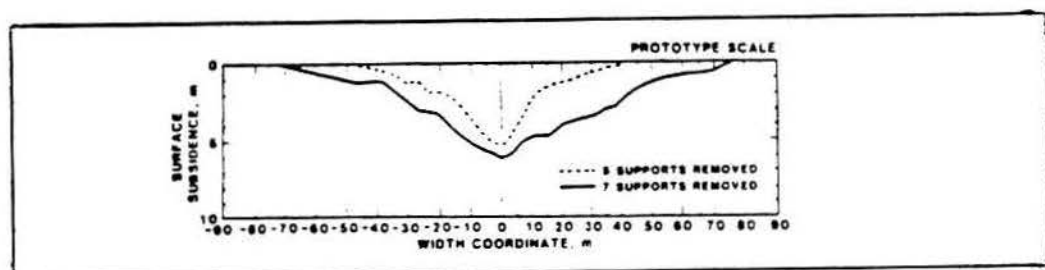


Figure 5. Centrifuge subsidence profiles [4].

With the final increase of the cavity width to 106.7 m (a total of 7 supports removed), many new cracks were formed and/or opened (see Figure 6). The subsidence trough extended to a width of 133 m with the maximum subsidence increasing slightly to 6.10 m (see Figure 5).

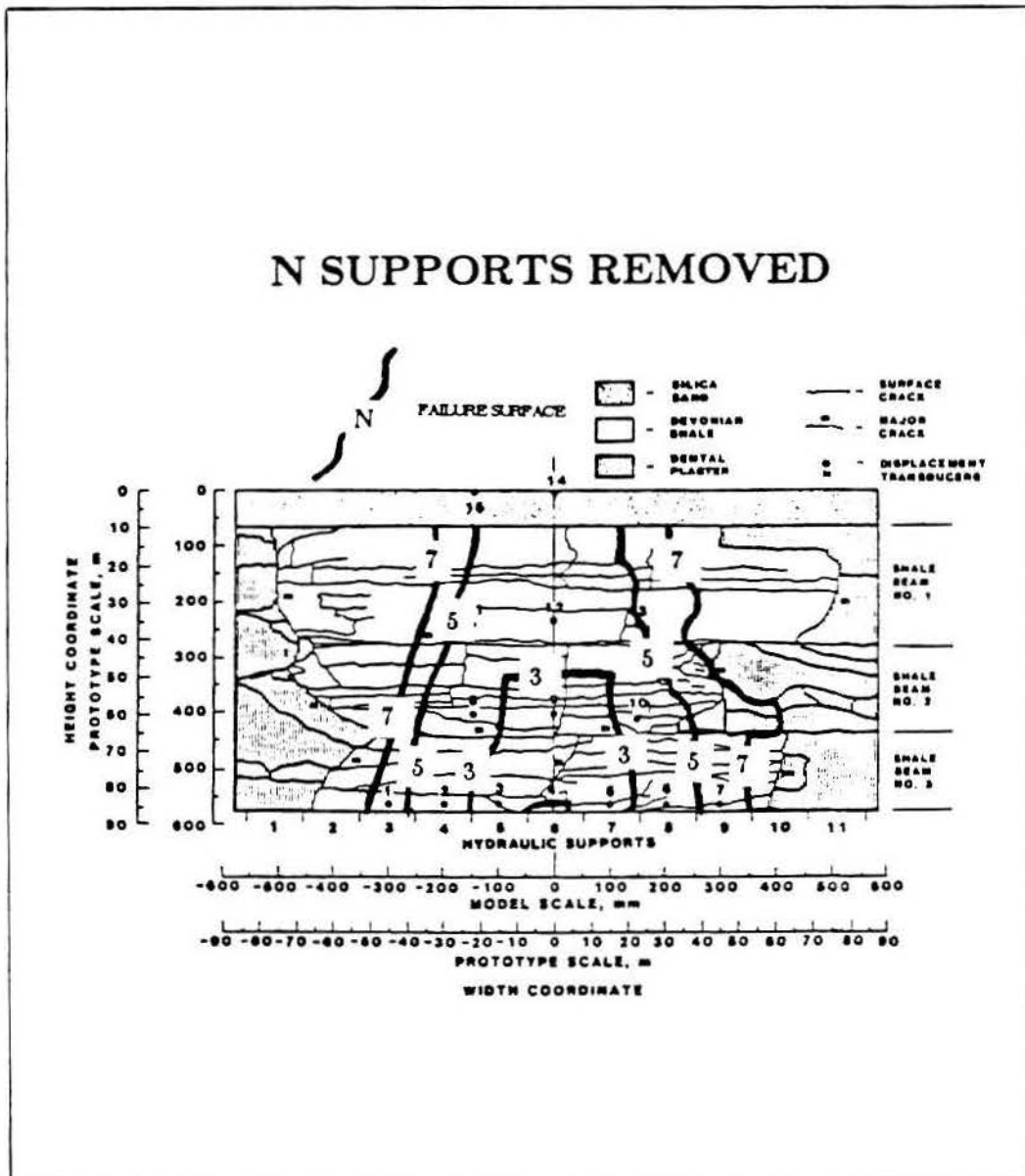


Figure 6. Failure progression of centrifuge model [4].

Finite Element Models

Equipment used. The numerical simulation of the mine was performed using SCRUBS.BYU on a VAX 11750 computer. Computer programs used in addition to the finite element code included a mesh generator (QMESH.BYU), a pre-processor (PRESCRUBS.BYU), an interface for the output (INTERFACE), and the graphics package (MOVIE.BYU). A short description of each of these programs and their use is contained in this section (See also Figure 7).

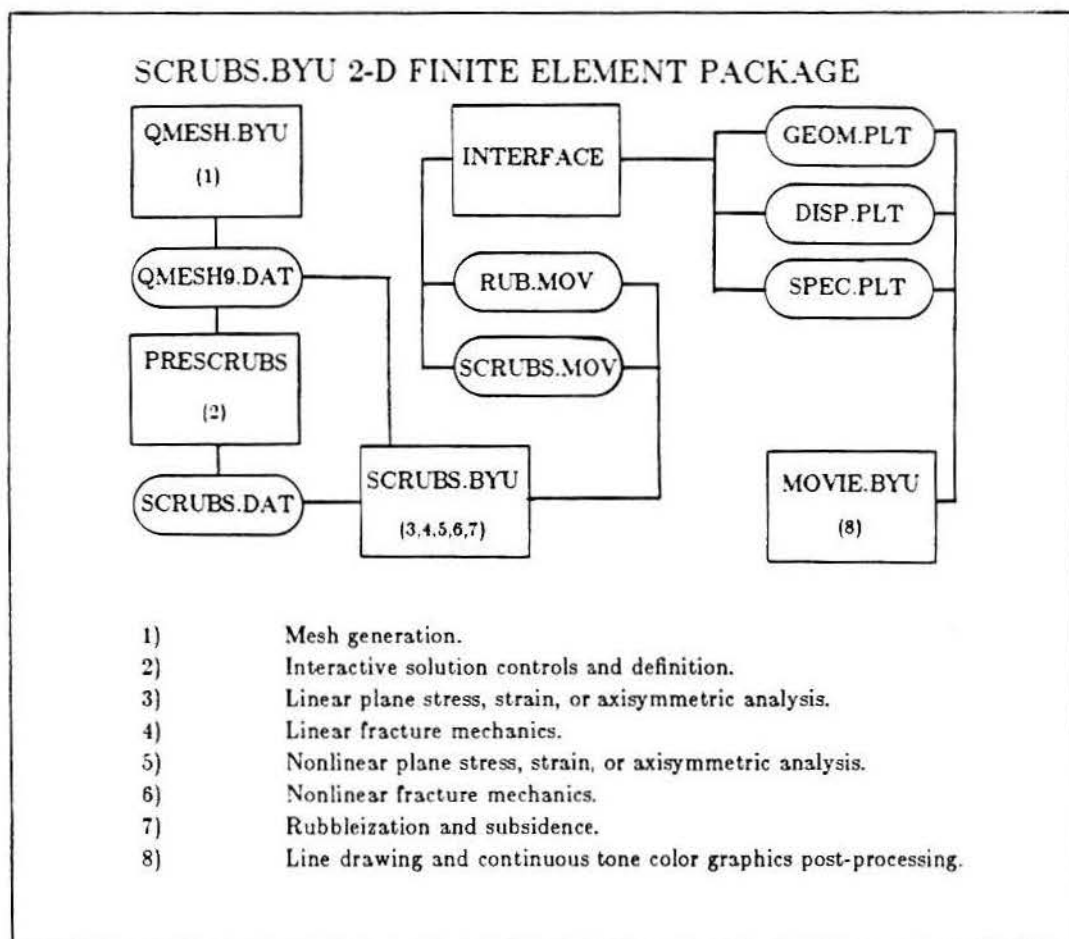


Figure 7. Program flow and capabilities [9].

Description of the various models. Six numerical models were used to simulate the response of the shale overburden; two different meshes were generated, and each mesh was used with three different combinations of jointed media continuum/discrete slip planes characteristics. The first mesh used the major vertical and horizontal fracture lines as determined in the Sandia test as region boundaries. This mesh approximated these major fracture lines as closely as possible (see figure 8). The horizontal joints were straight, but the vertical joints were not, which introduced some difficulties in constructing a uniform elements. As a result, this mesh was labeled the "skewed mesh." The second mesh approximated the major fracture lines with strictly vertical and horizontal boundaries. This resulted in a much more uniform mesh (see figure 9), and was labeled the "straight mesh."

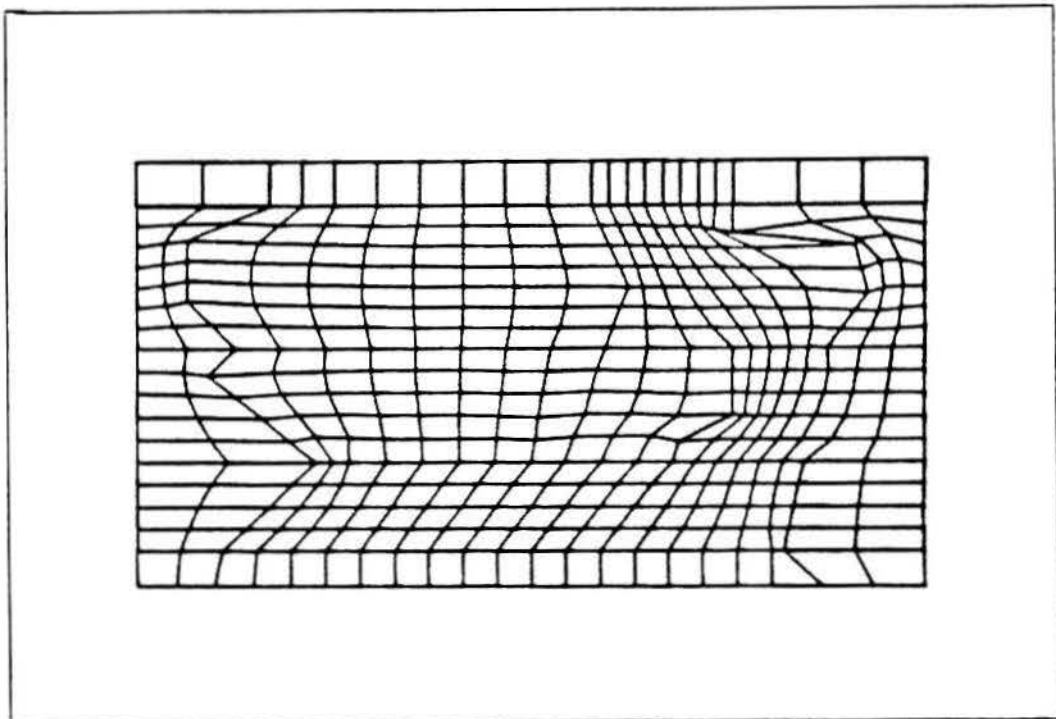


Figure 8. The skewed mesh.

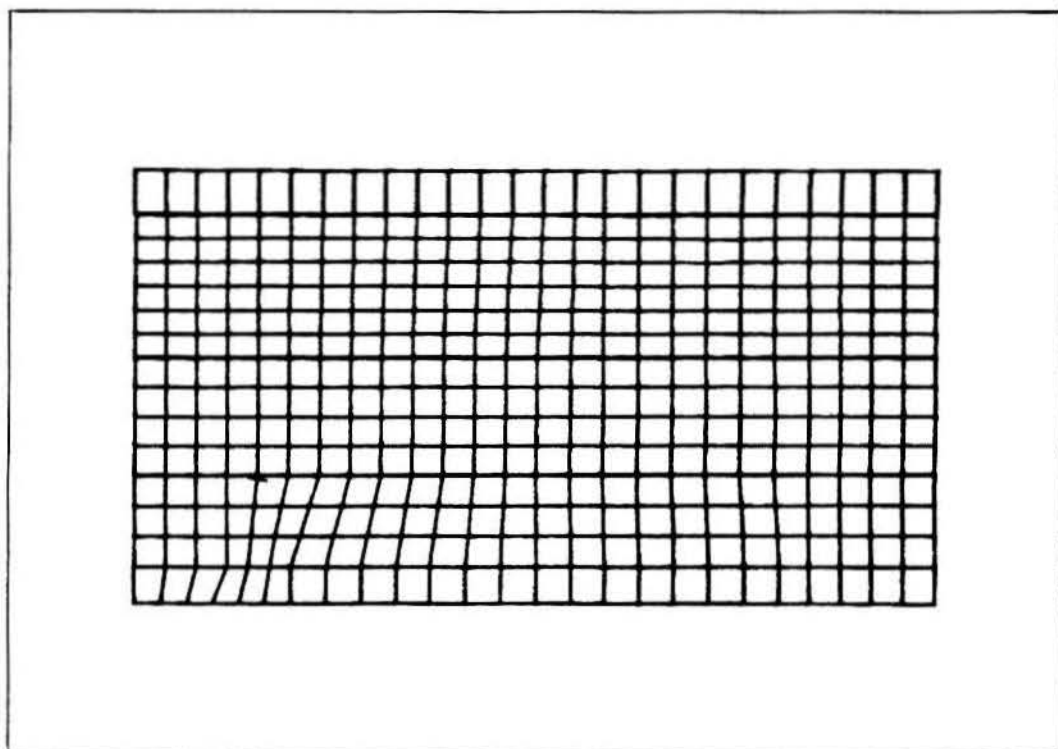


Figure 9. The straight mesh.

As stated, three different combinations of discrete slip plane/jointed media continuum were used with each mesh. The fracture combinations were:

- 1) Horizontal slip planes only (modeling of major slip planes).
- 2) Jointed media continuum only (modeling of minor fractures).
- 3) Combined jointed media and horizontal slip planes.

Construction of the meshes. Included in the mesh generation were the assignment of regions, material types, boundary conditions, application of smoothing algorithms to obtain even meshes, and renumbering of the mesh for more efficient use in the finite element code. The final meshes each contained 14 regions of four different materials (refer

to Figures 8 and 9). There were a total of 462 elements and 506 nodes in the skewed mesh, and 375 elements and 416 nodes in the straight mesh. The number of elements in the mesh was kept as low as it was judged possible and still maintain reasonable accuracy and reasonable computation time. Each potential slip plane required a unique flag to be set in QMESH.BYU. The mesh parameters, once processed by QMESH.BYU, were stored in a binary file named QMESH9.DAT to be used later by SCRUBS.BYU. Anyone not familiar with the operation of QMESH.BYU should consult Reference 8.

The problem parameters were all input using PRESCRUBS.BYU, the preprocessor for SCRUBS.BYU. These problem parameters included the material properties, the stress/strain field used, the yield condition, the element type, the number of gauss points to use in integration, the mining data, the convergence criteria, and the loading conditions. A state of plane strain was used throughout. Elasticity was assumed, the Von Mises failure criteria was chosen, gravity loads were applied, and linear isoparametric elements were integrated using four gaussian points per element. The rubbleization, slip plane, and jointed media characteristics were activated or deactivated by changing the corresponding flag in PRESCRUBS.BYU. The output from PRESCRUBS.BYU was stored in the file SCRUBS.DAT.

Model dimensions. The outside dimensions of both meshes were the same as those of the centrifuge model. The length was 112.2 cm, the height was 624.5 cm (from the bottom of the supports to the top of the sand), and the thickness was 20.2 cm. The steel elements had the same

depth, 51 cm, as the supports in the Sandia model. The 14 interior steel elements were defined to have exactly one half the width of the Sandia supports, 51 cm, so that two elements were equivalent to one Sandia support.

Materials modeled. Four materials were modeled in the numerical simulation. The four modeled materials were 1) silica sand, 2) Devonian shale, 3) dental plaster, and 4) steel. The silica sand, dental plaster, and Devonian shale were set up the same as in the centrifuge model. Steel was used for the material of the supports at the bottom of the model.

Material properties which were needed for the numerical simulation but not available from the Sandia centrifuge test information were determined from soil mechanics texts and judgement. A consistent system of units was used of centimeters, seconds, grams, and dynes for all measurements and quantities. For silica sand, a modulus of elasticity of 17 MPa ($170(10^6)$ dynes/cm²), a Poisson's Ratio of 0.3, and a density of 1.25 g/cm³ were used. For both the shale and the plaster properties of 69 GPa ($690(10^9)$ dynes/cm²) for Young's modulus, 0.29 for Poisson's Ratio, and 2.64 g/cm³ for the densities were used. For the steel, the modulus of elasticity was 200000 N/m² ($20(10^{11})$ dynes/cm²), Poisson's Ratio 0.3, and the density 7.8 g/cm³.

To use the rubbleization, slip plane, and jointed media capabilities of SCRUBS.BYU, additional material properties were required. Properties of the rubbleized material were set the same as for the shale. The

slip material was given a bulk modulus of $3.45(10^{11})$ dynes/cm² and a shear modulus of $3.45(10^{10})$ dynes/cm². The jointed media was given a fracture spacing of 10 cm and a slip modulus of 10^7 dynes/cm².

All materials were given a very high cohesion of 10^{20} dynes/cm² and a hardening modulus of one. The cohesion was set arbitrarily high so that the materials failed while still in the elastic range. The hardening modulus was arbitrarily set at one as it is not used in this formulation.

Failure condition. The failure condition was chosen to be when either the horizontal or vertical stress exceeded the failure stress. This condition is a user-supplied option in the subroutine FTEST (see appendix E). The failure stress for each model is noted in each figure caption (see Figures 10-15). The bulking parameter for rubbleization was set at 0.98 for all materials. The failure stress and the bulking parameter were set in PRESCRUBS.BYU. These choices gave generally good results, and were chosen for that reason.

The finite element program SCRUBS.BYU was used to analyze the model. The model was initially subjected to a gravity load of 150 gravities (1471.5 cm/s^2). The subsidence of the material under a gravity load was then determined using basic finite element techniques. This stabilizing step was called the first time step. Supports were removed in four additional time steps, with the center one (two elements) removed in the second time step and two neighboring supports (four elements) removed in each successive step, until seven supports (fourteen elements) had been removed.

The results. Results of the simulation run were stored in three files: (1) SCRUBS.LIS contained the parameters and the progression of failure, (2) SCRUBS.MOV contained the geometry, displacements, and stresses of the final mesh, and (3) RUB.MOV contained the rubbleized incremental geometry. SCRUBS.LIS contained the results in a readable form, while SCRUBS.MOV and RUB.MOV were binary files. Using an interface to the graphics package MOVIE.BYU, final subsidence profiles and rubbleized incremental profiles could be obtained from. correspondingly, SCRUBS.MOV and RUB.MOV.

Comparisons

The failure progression of the physical centrifuge simulation can be seen in Figure 6. This progression was used as a standard from which the numerical simulations were adjusted and refined in an attempt to match, and to which the results were compared.

The different meshes seemed to have some effect on the results. There was a tendency for the failure in the skewed mesh to tilt in the direction of the skewed elements, and in the straight mesh the failure would often pipe to the surface long before the failure in the skewed mesh would reach the surface. This piping is evident in both the jointed media and the discrete slip plane straight mesh models, although in slightly different forms. The centrifuge model had more of a pyramid failure form. The tilting of the failure in the skewed meshes did not seem to alter the overall pattern of failure, which was also pyramid in form.

In the jointed media straight mesh model (see Figures 10 a-d), piping to the surface occurred at the second time step (first support removed). After this initial piping, the failure seemed to follow the same general pyramid pattern of the centrifugal failure (and the skewed meshes).

In the jointed media skewed mesh model (see Figures 11 a-d), failure occurred in pyramid form, with the time steps favorably comparable to the progression which had occurred in the centrifuge model, including the formation of a failure arch, failure to the surface occurring at the fourth time step (5 supports removed), and a wide subsidence trough at the end.

In the discrete slip plane straight mesh model (see Figures 12 a-d), no piping occurred until the third time step, but failure to the surface was then direct for all the remaining time steps. The resulting trough was comparable to the physical model, but there was no formation of a failure arch at any time.

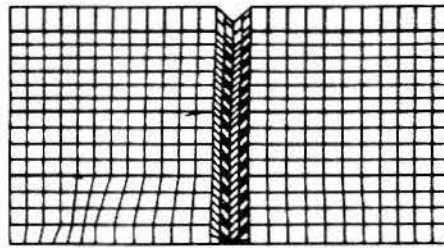
The discrete slip plane skewed mesh model was similar to the jointed media skewed mesh (Figures 13 a-d). The failure included a pyramid form, the formation of a failure arch at the third time step, failure to the surface at the fourth time step, and the final wide subsidence trough.

The combined straight mesh (Figures 14 a-d) resulted in a failure progression almost exactly like that of the jointed media straight mesh. A pipe failure to the surface occurred in the second time step, then a pyramid failure occurred from time step three to the final subsidence trough.

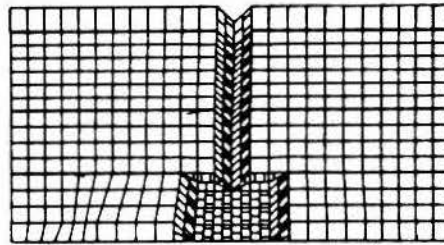
The combined skewed mesh (Figures 15 a-d) resulted in failure progression which was somewhat between that of the jointed media skewed

model, and interestingly, the jointed media straight model. Piping failure occurred, although not quite as drastically or sudden as in the straight meshes. The final subsidence trough was comparable to the others.

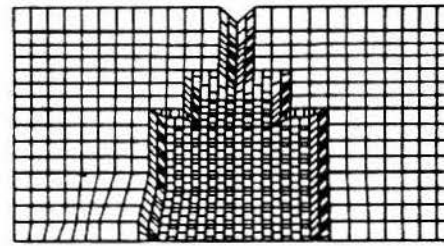
Some interesting variations of these numerical models are provided in appendix D.



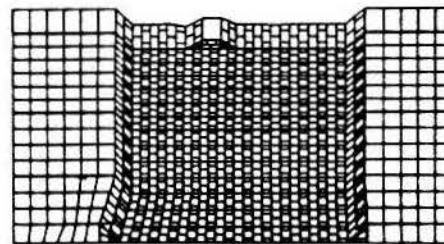
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b. Three supports removed.

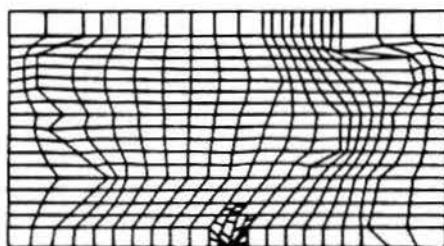


c. Five supports removed.

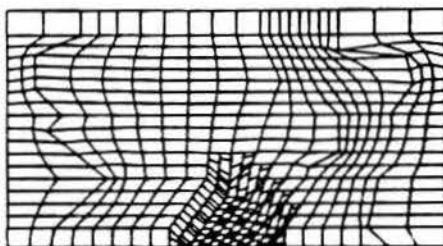


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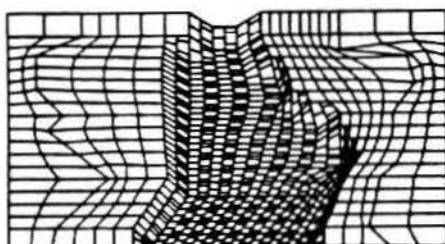
Figures 10 a-d. Jointed media straight mesh. Failure stress set to $0.1 \cdot 10^7$ dynes/cm² (tension).



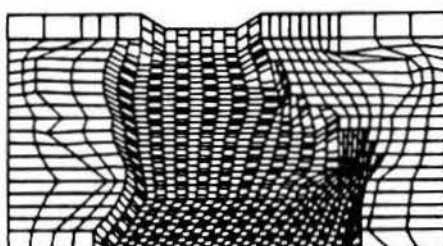
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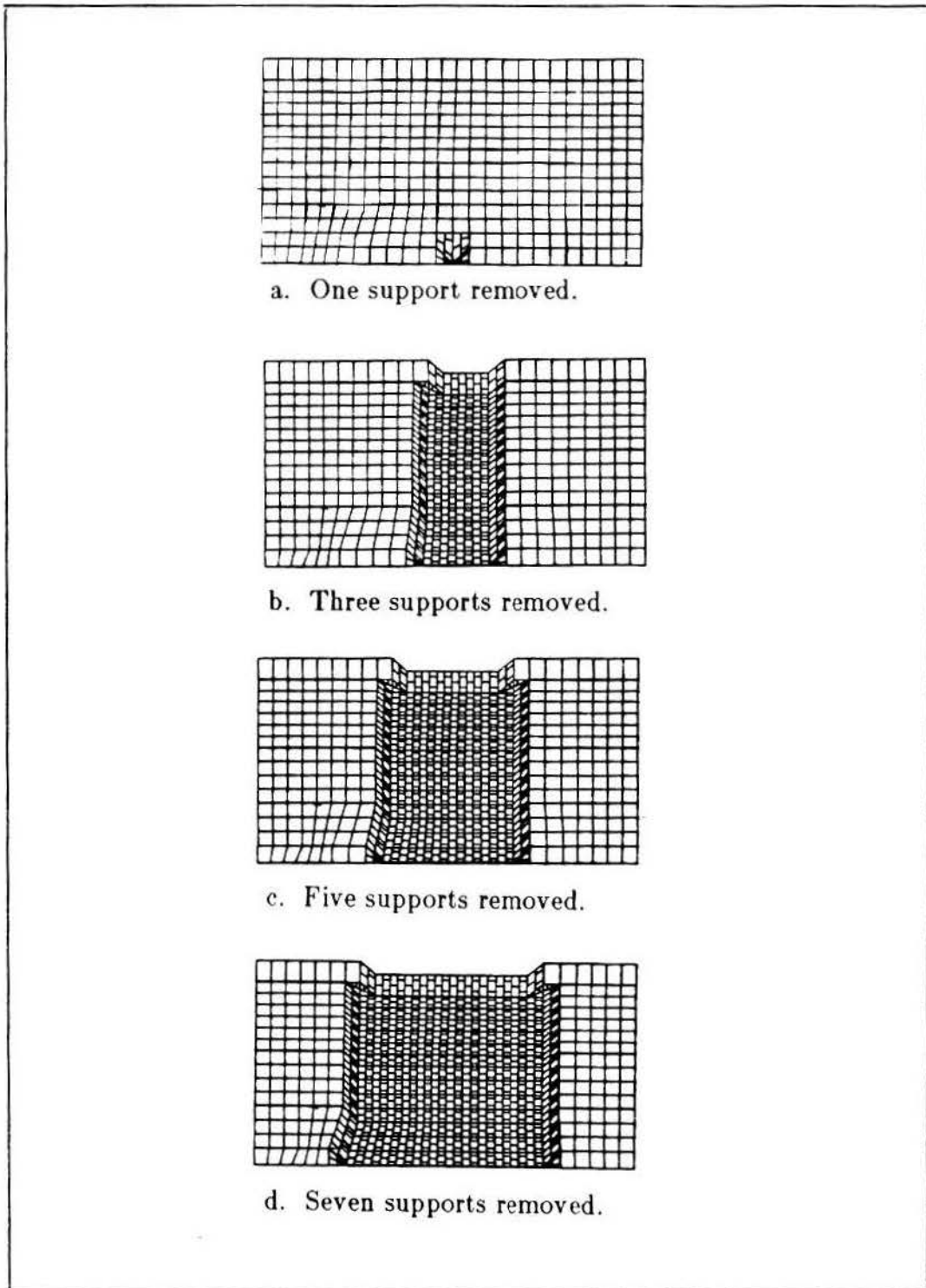


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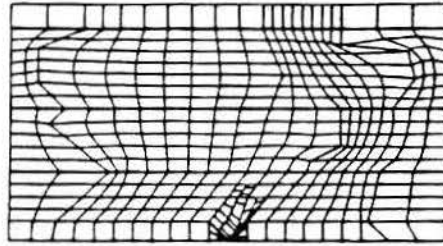


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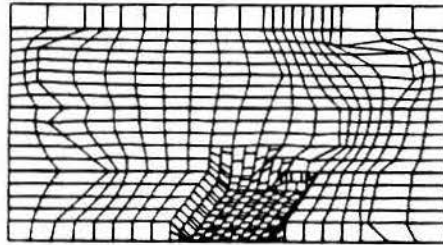
Figures 11 a-d. Jointed media skewed mesh. Failure stress set to zero.



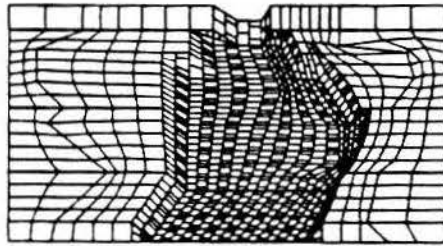
Figures 12 a-d. Discrete slip plane straight mesh. Failure stress set to 0.1×10^5 dynes/cm² (tension).



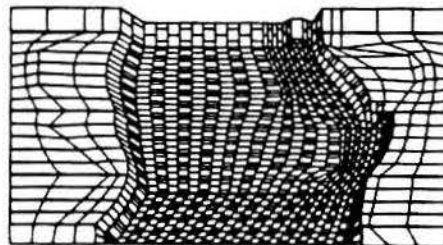
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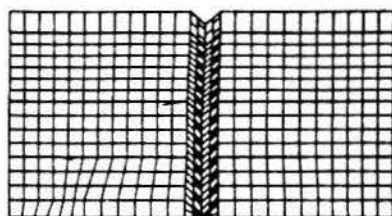


c. Five supports removed.

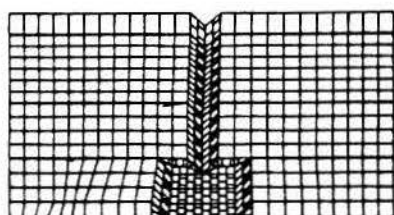


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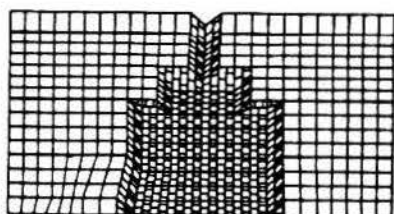
Figures 13 a-d. Discrete slip plane skewed mesh. Failure stress set to zero.



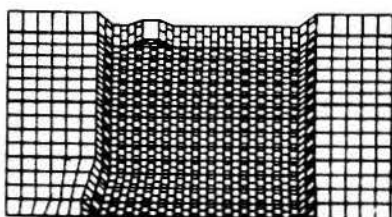
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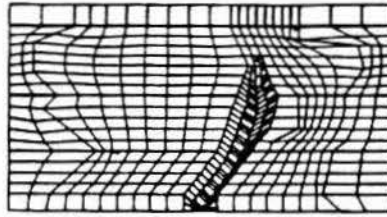


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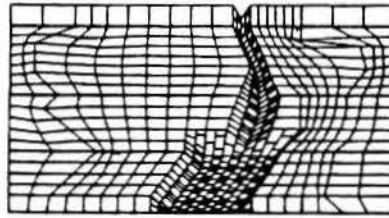


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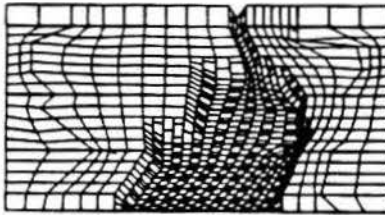
Figures 14 a-d. Combined straight mesh. Failure stress set to $0.1 \cdot 10^7$ dynes/cm² (tension).



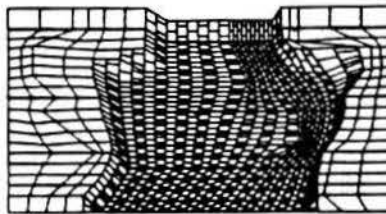
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d. Seven supports removed.

Figures 15 a-d. Combined skewed mesh. Failure stress set to 0.1×10^7 dynes/cm² (tension).

CHAPTER FIVE

CONCLUSIONS AND RECOMMENDATIONS

Knowledge of fracture and joint modeling in rock mechanics problems has been increased by the contribution of this thesis. Modeling fractures as either low shear element slip planes, jointed media continua, or as a combination of both are all valid approaches. Since many geologic materials contain both faults, which are better modeled by discrete slip planes, and rock mass with somewhat regular joint spacing, which is best modeled by continuum theory, it would make sense at times to combine them. In any case, it is possible for the stress analyst to be able to choose between the two models, or use the combination of both.

Two important characteristics of the simulations were closely considered: first, the failure sequence, and second, the final subsidence troughs. The general failure progression of all models in the mine subsidence problem were very similar. This would seem to indicate that all the models used were valid. To some extent, this success was dependent on an understanding of the properties and geometry of the problem. The second important characteristic considered, the subsidence troughs, were not always similar. The maximum displacements of the troughs were comparable, but the trough edges were not. As noted in chapter four, the physical centrifuge model resulted in gradually sloping trough edges, while the numerical simulations contained very steep trough edges. This is caused

by a weakness in the numerical stiffness formulation, which has difficulty in modeling the flow of the surface sand with solid finite elements. This difficulty is also indicated by the presence of isolated non-failed elements at the surface (refer to figures 10d, 13d, and 14d).

Problems and Solutions

The two different mesh types, skewed and straight, resulted in two consistent variations in the failure. First, the direction of failure was influenced, though only slightly, by the mesh type. The skewed mesh showed a tendency to fail diagonally, along the skewed elements, rather than in a more regular, vertical pattern like the straight mesh. The tendency for the skewed mesh to fail slightly diagonally implies that the finite-element formulation has some difficulty with the uneven elements. The failure was still quite good, however, and the final results seemed to be as good as those of the physical simulation. Second, the general form of failure varied between mesh types. The skewed mesh resulted in a pyramid form of failure, while the straight mesh contained more of a piping form. The varying forms of failure seem more pronounced than the slight directional tendencies.

For this problem, the skewed mesh seemed to be slightly more accurate. This is most likely due to the presence of major not-quite vertical slip planes in the centrifuge model (refer to Figure 4). While we were not able to use vertical slip planes (there is a problem with combining both vertical and horizontal discrete slip planes in a single model in

SCRUBS.BYU), the skewed mesh introduced diagonal failure tendencies. The diagonal failure may have reduced the piping failure, as the piping usually occurred vertically.

We used the least number of elements we could and still get results that did not seem effected by the element sizes. The run time for this kind of formulation and the size of the problem seemed reasonable. In our formulation, linear elements were used, whereas quadratic elements may have given better results. If quadratic elements were needed, fewer elements would be used, but the increased computational requirements for the quadratic formulation would offset the lowered computational requirements for the fewer elements.

As previously mentioned (chapter four), the failure stress was set at zero for the jointed media and discrete slip skewed meshes, at $0.1 \cdot 10^7$ dynes/cm² for the jointed media straight mesh model and the combined models, and at $0.1 \cdot 10^5$ dynes/cm² for the discrete slip plane straight mesh model. This criteria seemed to give the best results. Other criteria attempted included the separate horizontal and vertical stresses by themselves and the maximum principal stress. More study of the failure stress possibilities would be useful.

Although at present the use of the low shear element capability is currently limited to elastic deformations along the plane of slip, the incorporation of a plastic failure model for the slip plane material is possible. The Drucker-Prager yield criteria [6] would be especially applicable for this purpose and would be recommended for any further study in this area.

It should be noted here that the jointed media formulation in SCRUBS.BYU at present is only valid for a plane strain or axisymmetric condition. In the mine model, a plane strain assumption was made because of this, even though a plane stress situation may have been more realistic.

As previously mentioned, the poor profile of the subsidence trough at the edges in our numerical simulations were most likely a result of an incorrect stiffness formulation for the sand.

Summary

A summary of the recommendations made above are listed as follows:

- 1) Test the effectiveness of using quadratic elements in a model.
- 2) Study in detail the different failure criteria, and determine those which would be most accurate and in which situations.
- 3) Apply the Drucker-Prager yield conditions to a model in which the required parameters are available.
- 4) Do a plastic analysis of a subsidence problem, preferably using the Drucker-Prager yield condition.
- 5) Adjust the jointed media formulation for plane stress states.
- 6) Correct the stiffness formulation to allow for sandy materials.
- 7) Correct SCRUBS.BYU to allow combined horizontal and vertical discrete slip planes in a single model.

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APPENDICES

APPENDIX A

SCRUBS.BYU SUBROUTINE MAP AND DESCRIPTIONS

The following appendix provides subroutine descriptions and subroutine maps for the finite element code SCRUBS. The subroutine descriptions (the main is included) are listed alphabetically.

The subroutine descriptions:

- AUX: Creates coordinate jacobian $\mathbf{0}$ interpolate X; YOCOJ(I,J), XY(1),XY(2) are respectively the coordinate jacobian matrix, the X coordinate and the Y coordinate.
- BSUB: Conducts systematic back-substitution and solves for boundary displacements and reaction transformations.
- CONRUB: This subroutine checks for rubble/continuum interaction and computes the hydrostatic force in the rubble if a continuum element is in contact.
- CONVG: Determines the percent convergence.
- CORNN: This subroutine determines if node IX is a corner node.
- DATALST: Writes title, line numbers for files (presently not used).
- DROP: Checks for elements about to fail and adjusts geometry for failure.
- FLAGE: Makes impact node calculations and new node assignments.

- GAUSSP: Initializes gaussian constants.
- GDATA: Reads geometrical data, boundary conditions, and gaussian integration data.
- INVAR: Calculates invariants and sets the yield stress accordingly.
- JOINTM: Calculates incremental stresses and residuals.
- LDATA: Reads load control data.
- LINEAR: Helps to update stress and strain.
- MAKEEL: This subroutine adds a new element to the connectivity containing corner nodes I and K.
- MOD: Forms D matrix for different stress/strain cases.
- NEWNOD: This subroutine finds the new node number of node N.
- NFLOW: Calculates the amount of yield that results.
- NODEXY: Sets x,y values of new intermediate coordinates of quadratic or cubic elements.
- OUTPUT: Writes displacements, reactions, stresses and strains to SCRUBS.LIS
- PDISP: Initializes the displacement vector according to number of loading increments.
- READF: Reads the mining parameters: which elements, and when.
- REMLD: Removes gravity load from failed elements.
- RESIDUE: Determines element failure, rubbleization, and residual.
- RESOLV: Updates stiffness equation solution.
- ROSB: Adjusts stiffness matrix.
- ROTATE: This subroutine rotates 2D stress or strain vector by angle THETA.
- RUBDRAW: Writes the rubble file RUB.MOV.

- RZBASE: This subroutine finds the base R and Z dimensions for fall calculations.
- SCRUBS: Program designed to compute the failure, collapse, and resulting subsidence of geologic materials. Performs elastic/plastic analysis of plane stress/strain and axisymmetric problems for linear, parabolic, and cubic elements.
- SCRUBSV: This subroutine opens and closes the files used by the main program, SCRUBS.
- SET: Sets element spacing and determines bandwidth and resulting stiffness storage required.
- SFR: Sets shape functions according to element type.
- SLIPP: This subroutine reads the slip plane data, adjusts the geometry and elements accordingly, and then records the adjusted information.
- SOLVE: Solves stiffness equations, modifies loads.
- STIFM: Forms and updates stiffness matrix, and calculates body forces.
- SWITCH: Switches coordinates.
- TFORM: Makes adjustments for inclined boundary situations.
- ZONE: This subroutine reads meshing cards as done in the CHILES2 program. It is provided here only as an interim capability. This subroutine is called by the main program.

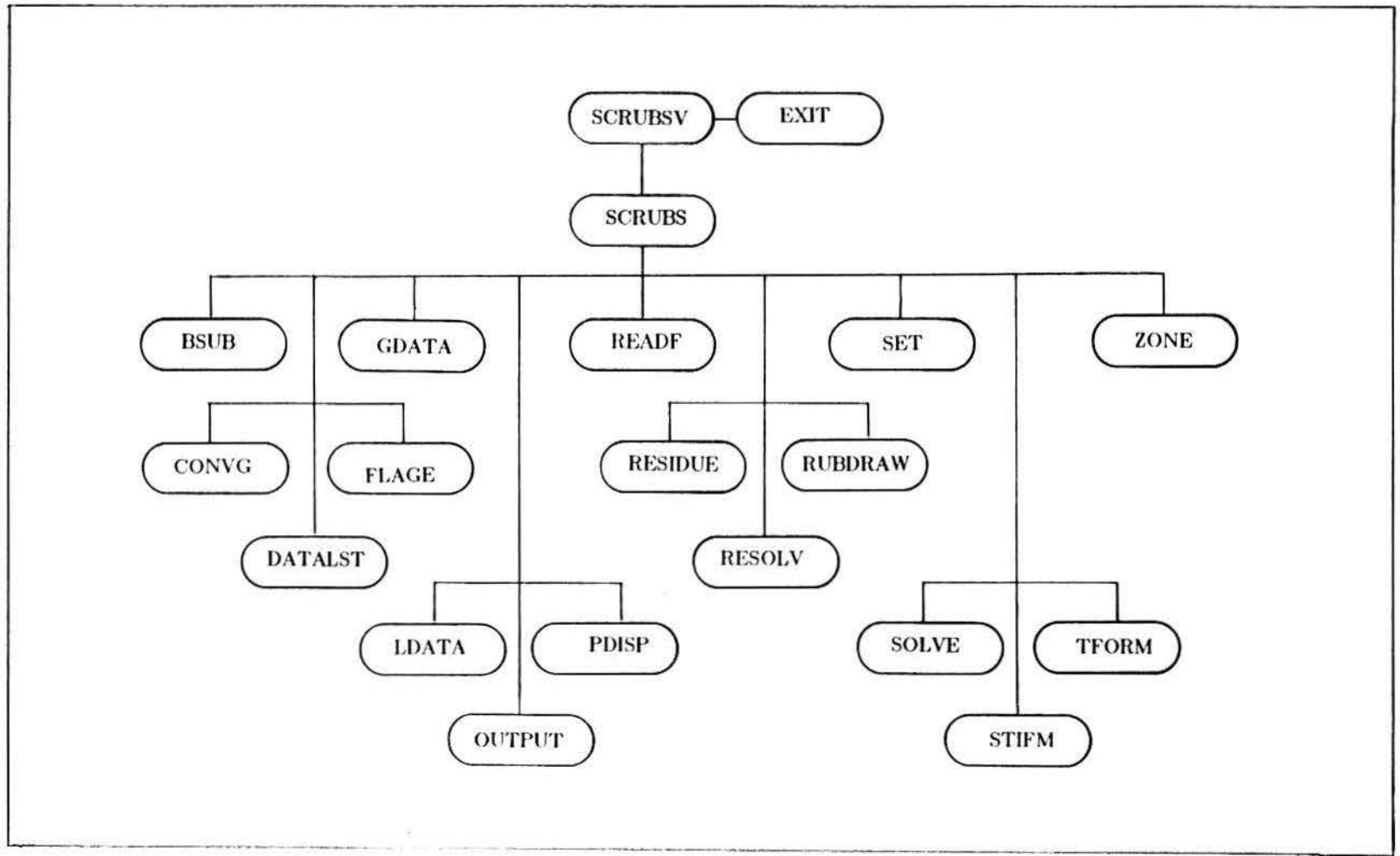


Figure 16 a. Subroutine map -- main.

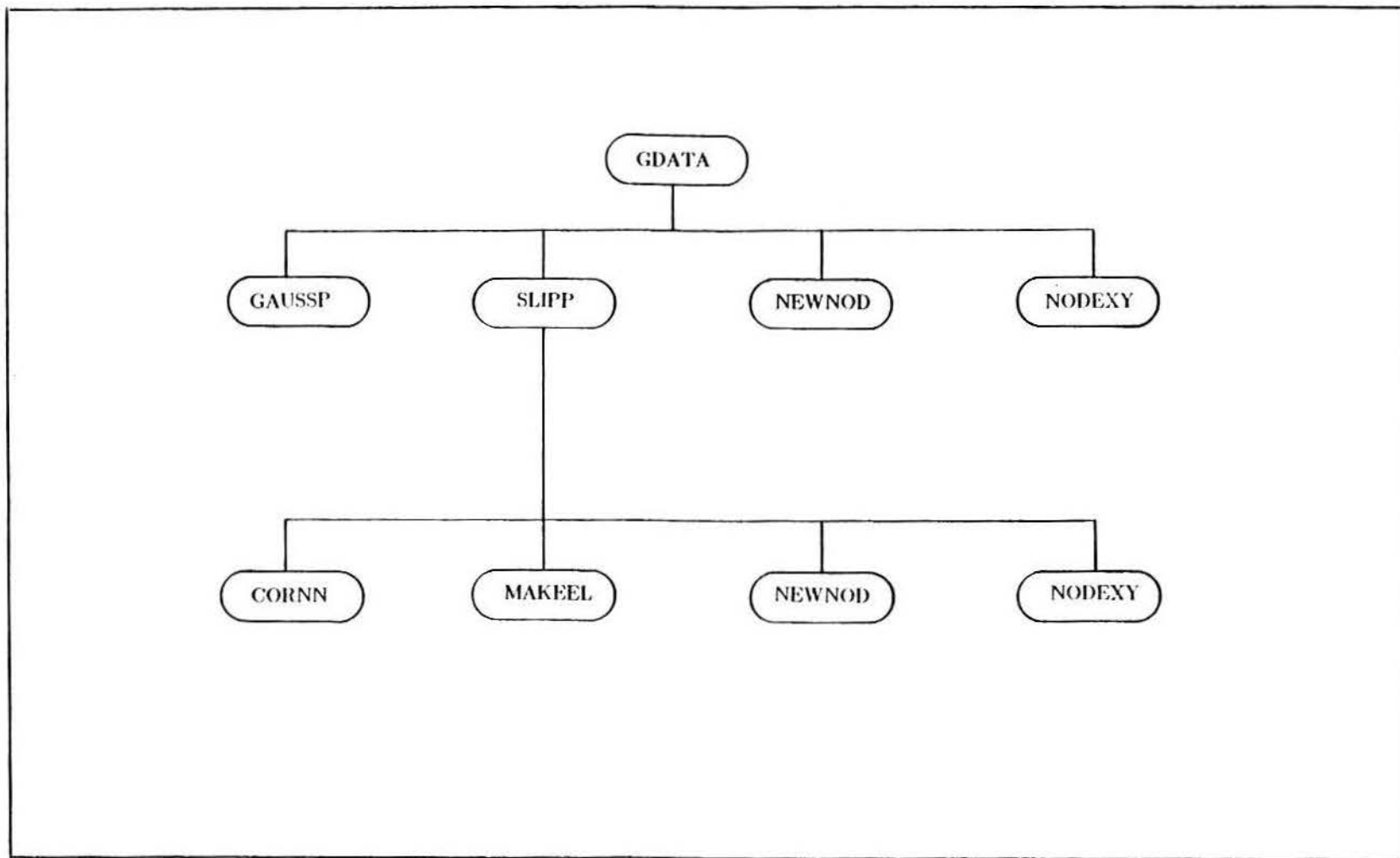


Figure 16 b. Subroutine map -- GDATA.

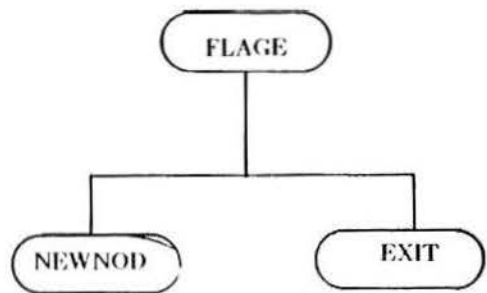


Figure 16 c. Subroutine map -- FLAGE.

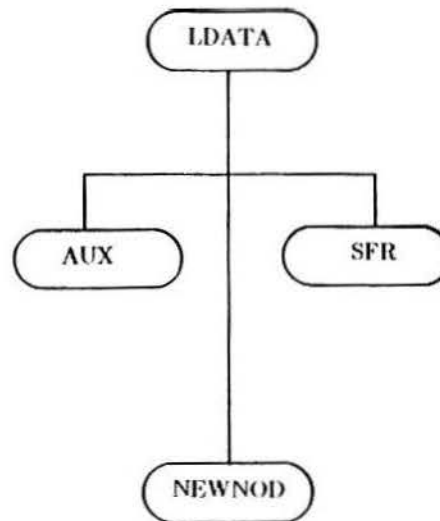


Figure 16 d. Subroutine map -- LDATA.

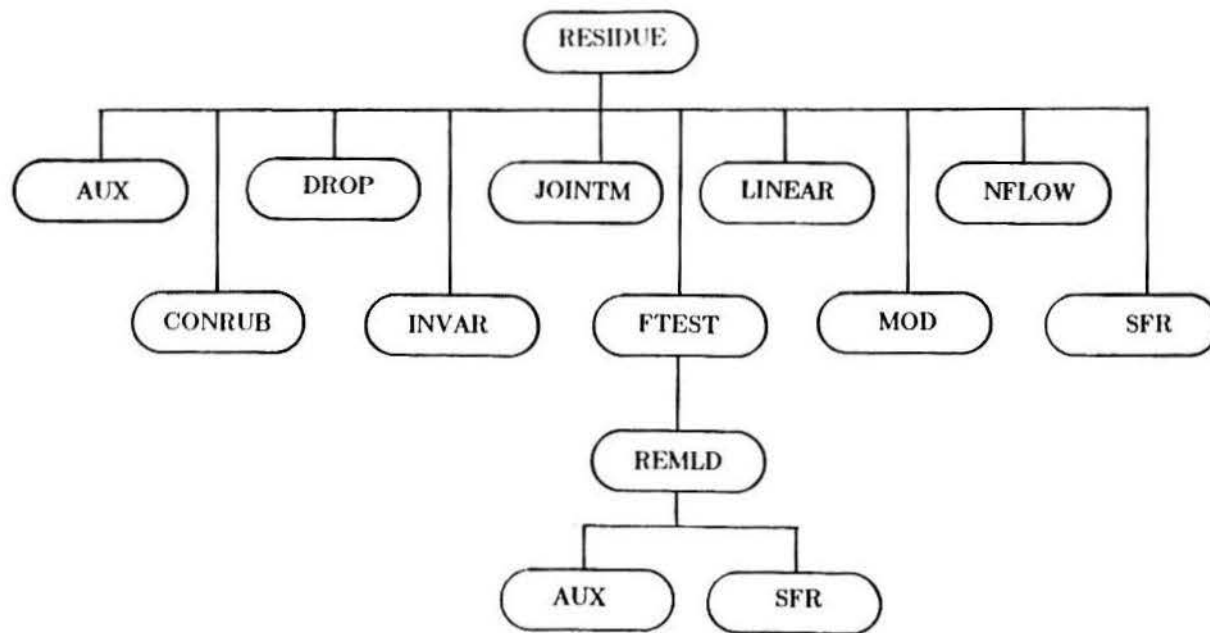


Figure 16 e. Subroutine map -- RESIDUE.

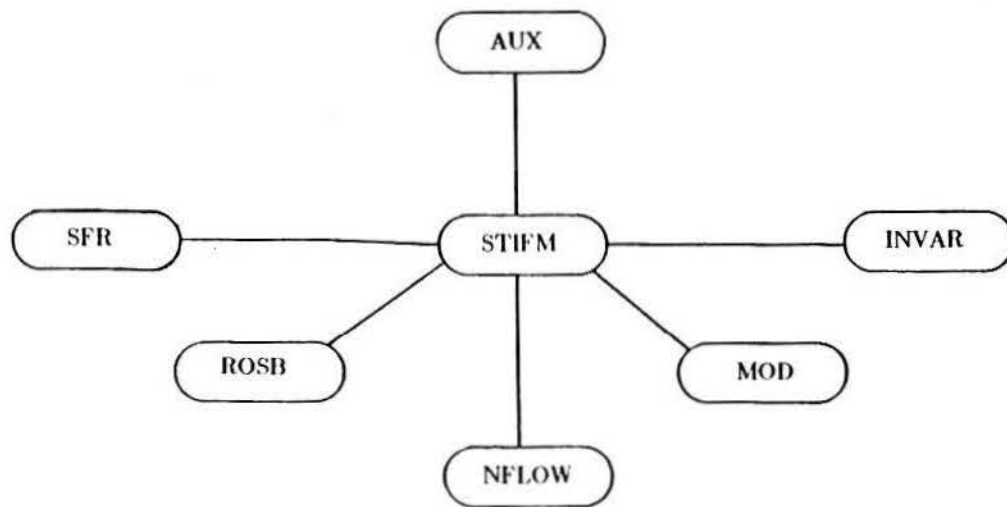


Figure 16 f. Subroutine map -- STIFM.

APPENDIX B

SCRUBS.BYU USER'S MANUAL

The following appendix provides: first, a short description of the program SCRUBS and its capabilities; second, detailed descriptions of the three files which are read by SCRUBS and the four files that are created by SCRUBS.

SCRUBS.DOC

SCRUBS is a FORTRAN computer program designed to compute the failure, collapse, and resulting subsidence of geologic materials. The uniqueness of this program is its ability to model rubble formation and collapse in a continuum, as opposed to a discrete, sense. Both pre and post failure aspects of particular problems are treated. SCRUBS is a nonlinear finite element program which is capable of determining the deformation and state of stress in plane and axi-symmetric bodies. Pre and post failed material properties may be elastic-perfectly plastic, or elastic work hardening. Linear, quadratic or cubic isoparametric elements are used to represent the region and its boundary. Four different yield conditions can be imposed on the materials: von Mises, Tresca, Drucker-Prager, or Beltrami. Three different algorithms for solving the nonlinear discretized equations are available. They are: 1) a constant stiffness initial stress method, 2) a two step process where the stiffness matrix is updated on the second iteration of an otherwise initial stress process and 3) a regular tangent stiffness method. A check on force residuals is used to evaluate convergence for any load increment.

Three specific files are read and four other specific files are created by SCRUBS. The three files that are read have the following names and functions.

QMES9.DAT = Mesh information as created by QMESH.BYU
 SCRUBS.DAT = SCRUBS input data
 SCRUBS.RST = Restart file

The four files that are created have the following names and functions.

SCRUBS.MOV = Displacement, stress and strain data at each load step
 RUBBLE.MOV = Element rubbleization plot file
 SCRUBS.RST = Restart file
 SCRUBS.LIS = List of failure sequence

The contents of these six files are now explained in detail.

QMESH9.DAT

This is the standard QMESH.BYU renumbered file that contains the two dimensional finite element meshing information. It consists of five records written in blocked binary form. The content of these records is as follows.

- Record 1. (8 words) A comment, packed 4 characters per word. This comment is from the COMMEN card in the QMESH input.
- Record 2. (4 words)
 KKK, the number of elements in the mesh
 NNN, the number of nodes in the mesh
 NEF, the number of words in the boundary flag table
 MAXDIF, the maximum difference of node numbers for any element in the renumbered mesh
- Record 3. (2 x NNN words) the lists of nodes, in renumbered order; that is
 (X(N), Y(N), N=1, NNN),
 or
 (R(N), Z(N), N=1, NNN)
- Record 4. (5 x KKK words) The list of elements:
 (N1(K), N2(K), N3(K), N4(K), MAT(K), K=1, KKK)
 where N1(K) to N4(K) are the node numbers for the K-th element in counterclockwise order, and MAT(K) is the material number of the K-th element.
- Record 5. (NEF words) the list of boundary flags and nodes, (IFLAG(I), I=1, NEF). Flags will be negated to distinguish them from nodes, and the corresponding node or list of nodes will follow each flag. If NEF=0, this record will not be written.
- End-of-File mark.

SCRUBS.DAT

This file contains the data necessary to define the problem to be analyzed. The groups of lines of this file and the specified format of each line follows.

GROUP 1 INTERNAL MESH GENERATION OR RESTART SPECIFICATION

Line 1. (4I5) Geometry parameters
 1 - 5 Number of elements (NEL) **
 6 - 10 Number of nodes (NODES)
 11 - 15 Number of load cards (NUMPC)

** NOTE if NEL = 0, omit remaining lines of the group and read total mesh from the input file QMESH9.DAT. If NEL = -1, the problem is to be restarted and the remaining lines of this group are omitted.

Line 2. (6I5) N,IX array
 1 - 5 Element number (N)
 6 - 10 Ith nodal point (IX array)
 10 - 15 Jth nodal point (IX array)
 16 - 20 Kth nodal point (IX array)
 21 - 25 Lth nodal point (IX array)
 26 - 30 Material number (MAT, IX array)

In general every element must be defined; but with the semi-automatic mesh generation feature, a minimum of one element per row need be input. For example, if element 10 is read with values I=12, J=13, k=24, L=23, and MAT=1, and the next element is read is element 15 with values I=23, J=24, K=35, L=34, and MAT=1, then element 11 would be assigned values 13, 14, 25, 24, and 1.

Line 3. (I5,F5.0,4F10.0)
 1 - 5 Nodal point number (N)
 6 - 10 Boundary condition code (CODE)
 11 - 20 Radial coordinate (R)
 21 - 30 Axial coordinate (Z)

In general, every nodal point must be defined, but since the program has a semi-automatic mesh generation feature, a minimum of two nodal points per row need be input and the intervening points will be assigned coordinates based on a linear interpolation procedure. For example, if nodal point 1 is the first point in a row with coordinates (2.5, 5.4), and nodal point 11 is the next point defined with coordinates (12.5, 10.4), then nodal point 2 will be located at (3.5, 5.9), etc.

GROUP 2 SOLUTION CONTROLS

Line 1. Problem identification
 1 - 5 Total number of problems to be solved in one run (NPROB)

Line 2. Title (20A4)
 1 - 72 Title of problem.

Line 3. Control data (16I5)
 1 - 5 Total number of nodes, NP (not more than 1000)
 6 - 10 Total number of elements, NE (not more than 1000)
 11 - 15 Total number of restrained boundary points, NB
 16 - 20 Total number of load cases/problem, NLD
 21 - 25 Number of d.o.f. per node, NDF
 26 - 30 Number of different materials, NMAT (not more than 10)
 31 - 35 Element type 1=linear, 2=parabolic, 3=cubic, NSER
 36 - 40 Number of gauss points for stiffness calc. NGAUS
 41 - 45 Solution algorithm, NALGO
 0 = elasticity only
 1 = constant stiffness
 2 = two step process
 3 = tangent stiffness
 46 - 50 Stress/strain type NFP
 0 = Plane strain
 1 = Plane stress
 2 = Axisymmetric problem
 51 - 55 Yield condition parameter, NYIELD
 1 = Mises
 2 = Tresca
 3 = Drucker-Prager
 4 = Beltrami
 56 - 60 Input stiffness control, NT
 0 = Number of elastically coupled nodes
 1 = input stiffness coefficients
 61 - 65 Flag for Rubble calculation, NL
 0 = No rubble
 1 = rubble
 66 - 70 Bandwidth (leave blank unless NT = 1)
 71 - 75 Number of coordinates per node NCORD (default 2)
 76 - 80 Number of gauss points for nodal force residual
 calculation and stress storage (MGAUS) result even for
 elastic solution MGAUS defaults for NGAUS.

GROUP 3 - MATERIAL PROPERTIES

Line 1. Material data (I10, 7F10.2) NMAT Lines, limited to NMAT= 10
 (Note, if NL = 1, card NMAT of this section must give
 unconsolidated rubble constants)

1 - 10	Material property number	N
11 - 20	Young's modulus	ORT(N,1)*
21 - 30	Poisson's ratio	ORT(N,2)**
31 - 40	Yield stress	ORT(N,3}
41 - 50	Hardening modulus	ORT(N,4}
51 - 60	Conical yield surface angle	ORT(N,5}
61 - 70	Thickness (leave as zero if NPP = 1)	ORT(N,6)

* A negative value in this position activates the low shear material description. In this case, the bulk modulus, K, is the absolute value of ORT(N,1), and the shear modulus, G, is the value of ORT(N,2)

** A negative value in this position activates the jointed media material description. For this case, one more line is read, for that material only, defined as follows: (2G12.5)

1-12	Fracture spacing	ORT(N,9)
------	------------------	----------

Line 2. Tabulated plastic stress-strain data
 (Only for materials with ORT(N,4) negative) (I10,F10.3)
 1 - 10 Number of tabulated strain points, NTAB
 11 - 20 Strain increment in percent strain, TABSTN(N)
 If ORT(N,4) > 0 these values default to 2 and 100 respectively.

Line 3. String of stress values, NTAB in number, one line required for
 each material (15F8.0)

1 - 8	Stress value for point 1
9 - 16	Stress value for point 2
17 - 24	Stress value for point 3
25 - 32	Stress value for point 4

:
 :
 :
 etc.

Line 4. String of strain values, NTAB in number, one line required for
 each material (15F8.0)

1 - 8	Strain value for point 1
9 - 16	Strain value for point 2
17 - 24	Strain value for point 3

:
 :
 :
 etc.

 **

GROUP 4 RUBBLEIZATION DATA (Omit this group if NL=0)

Line 1. Rubble material data, one line required for each material
 (I10,2F10.3)
 1 - 10 Material number,N
 11 - 20 Bulking parameter, ORT(N,7)
 21 - 30 Failure stress ORT(N,8)

Line 2. Minimum boundary coordinate (E10.3)
 1 - 10 Value of minimum Z coordinate for which element nodal
 points cannot pass through

Line 3. Material failure flag (I5)
 1 - 10 Failure flag, NRFF
 If this flag is non zero, a subroutine, READF is
 called. This subroutine defines a list of elements
 that can be removed (i.e.mined) on a particular
 increment. NRFF is the total number of increments
 allowed (10 max).

Line 4. Element mining data, omit if NRFF=0. (16I5)
 1 - 5 Number of elements to be mined on increment 1, NME(1)
 6 - 10 Number of elements to be mined on increment 2, NME(2)
 11 - 15 Number of elements to be mined on increment 3, NME(3)
 16 - 20 Number of elements to be mined on increment 4, NME(4)
 . . .
 . . .
 etc.

Note: Currently 16 is the maximum number of elements
 that can be mined on any given increment.

Line 5. Specified elements to be mined, one line required for each
 increment, NRFF total, omit if NRFF=0. (16I5)
 1 - 5 Element to be mined on this increment, MINES(INC,1)
 6 - 10 Element to be mined on this increment, MINES(INC,2)
 11 - 15 Element to be mined on this increment, MINES(INC,3)
 16 - 20 Element to be mined on this increment, MINES(INC,4)
 . . .
 . . .
 etc.

GROUP 5 BOUNDARY CONDITIONS

Line 1. Ancillary control data (16I5)
 1 - 5 Number of slip planes NNP
 6 - 10 Number of singular points NSING
 11 - 15 Number of boundary points NNBB
 16 - 20 Number of reactions to be summed MPR
 21 - 80 Indexing vector, automatic generation of NOPE array
 IB(I), I=1,12

Line 2. Boundary conditions (2I5,4F10.3) NNBE cards in ascending order
 of boundary nodes.
 1 - 5 Boundary node number NBC(I)
 9 u condition, 0=free, 1=fixed
 10 v condition, 0=free, 1=fixed
 11 - 20 Prescribed u displacement US(I,1)

21 - 30 Prescribed v displacement US(I,2)
 31 - 40 Boundary angle (if inclined) ANG(I)
 in degrees of X' axes from X.
 Positive when counterclockwise.

Lines 3 and 4 are repeated in NSP blocks, each block representing a new slip plane. If NSP = 0, omit lines 3 and 4.

Line 3. Slip plane control data (2I5,G10.3)
 1 - 5 Number of nodes on slip plane boundary NSLP(N)
 6 - 10 Angle flag of slip plane IANG(N)
 11 - 20 Thickness of slip plane STHIK(N)

Line 4. Slip plane node list (16I5) NSLP(N)/16 cards
 1 - 5 Node number
 6 - 10 Node number ISLP(I,1), I = 1,NSLP(N)
 11 - 15 Node number
 . . .
 . . .
 etc.

Line 6. Summed reaction nodes (16I5) (If MPR>0, otherwise omit)
 1 - 5 Node number
 6 - 10 Node number NOPR(I), I = 1,MPR
 11 - 15 Node number
 . . .
 . . .
 etc.

 * NOTE!!! THE FOLLOWING LINES OF *
 * GROUP 6 ARE REPEATED IN NLD BLOCKS *
 * EACH BLOCK REPRESENTING A NEW LOAD *
 * CASE *

GROUP 6 LOADING DEFINITION

- Line 1. Ancillary load control data (2I5)
 1 - 5 Initial stress counter ISTS (not used)
 6 - 10 Increment control (Note LDTYPE
 applies to both prescribed
 loads and displacements.)
 0 ... equal increments
 1 ... increments in proportion to total
 according to the fac array
 {X } = FAC {X }
 inc inc total
 2 ... independent set of loads for each
 increment
- Line 2. Loading title (20A4)
- Line 3. Loading control (8I5)
 1 - 5 NSTRS (Not used)
 6 - 10 Number of nodes where point loads are applied, NRE
 11 - 15 Gravity load flag, if nonzero read lines 5 and 6.
 16 - 20 Pressure load flag, if nonzero read lines 7, 8 and 9.
 21 - 25 Temperature load flag, currently not available.
 26 - 30 Centrifugal force flag, if nonzero read lines 5 and 6.
- Line 4. External Point load data (I10,3F10.3) NRE lines
 1 - 10 Node number
 11 - 20 Force in the X direction
 21 - 30 Force in the Y direction
- Line 5. Gravity and centrifugal force data (3F10.3) two or three lines
 if NRC + NRG > 0. Otherwise go to line 7.
 1 - 10 Angle of Y or Z axes from gravity axes, clockwise
 positive. THEDA
 11 - 20 Number of g's applied for this system
 (Only when NRC = 1) GRAV
 21 - 30 Angular velocity in radians/unit time
 (only when NRC = 1) ANGVEL
- Line 6. Density (8F10.3)
 1 - 10 Density for material 1 DENS(1)
 11 - 20 Density for material 2 DENS(2)
 .
 .
 .
- Line 7. Pressure element control, if no pressure loads (i.e. NRPRS=0)
 leave out and go to line 10. (8I5)
 1 - 5 Number of line elements on which pressures are applied
 LNE

Note each line 8 and 9 should be in sequence i.e., 8,1

9,1
8,2
9,2
etc.

Line 8. Element nodes with applied pressure, one line/element (4I5)

```

1 - 5 node 1      0-----0
6 - 10 Node 2    0-----0-----0
11 - 15 Node 3   0----0---0----0
16 - 20 Node 4   0----0---0----0

```

NOPL(I), I=1, NSFR+1

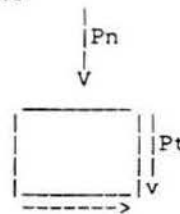
Node numbers on element pressure surfaces numbered in counterclockwise direction.

Line 9. Pressure data (8F10.3)

(a) same loads on each node of line element
1 - 10 Normal pressure (Pn)

11 - 20 Tangential pressure (Pt)

21 - 30 Zero



(b) Different loads at each node of line element Pn

```

1 - 10 Pn |
          | Node 1
11 - 20 Pt |
          |
21 - 30   |
          | Node 2
31 - 40   |

```

etc.

Return to 8 for next line element.

Note (a) does not apply for linear elements

Line 10. Load increment data (I5)
 1 - 5 Number of load increments NINC

Line 11. Load factor data (16F5.3)
 1 - 5 Multiplier for increment 1 FAC(1)
 6 - 10 Multiplier for increment 2 FAC(2)
 : :
 : :
 etc.

Line 12. Output control data (16I5) NOUT(I), I=1,NIC
 1 - 5 s o t s = 1st iteration write out indicator
 t = final iteration write out indicator

all printout is given up to and including s or t

s or t = 0 no output
 = 1 displacements at nodal points
 = 2 reactions at constrained nodes as well
 = 3 stresses at gauss points as well
 = 4 residual forces at nodes as well

6 - 10 NOUT(2)
 11 - 15 NOUT(3)
 : :
 : :
 etc.

Line 13. Iteration and convergence data (I10,F10.3)
 1 - 10 Maximum number of iterations NIT
 11 - 20 Convergence factor in percent CONFAC
 checks on $100 \times (\text{Sum of } | \text{force residuals} |)$
 divided by $(\text{Sum of } | \text{applied forces} |)$

GROUP 7 REDUCED PRINTOUT CONTROLS

Line 1. Print control (2I5)
 1 - 5 Number of elements to be printed. If negative,
 all elements printed.
 6 - 10 Nodal point output control, if zero, no nodes
 printed, if 1, all nodes printed.

Line 2. List of elements to be printed (16I5) (only if NELP > 0)
 1 - 5 element number to be output
 6 - 10 element number to be output
 11 - 15 element number to be output
 : :
 : :
 etc.

SCRUBS.MOV

This file is written in blocked binary form as follows:

Record 1. (22 words) the first twenty words are the title of the problem and the last two words are NE, the total number of elements, NP, the total number of nodal points, that is,

TITLE,NE,NP

Record 2. (2xNP+5xNE words) the first two lists are the nodal point coordinates, the next is the element connectivity, and the last is the element type.

(CORD(I,1),I=1,NP), (CORD(I,2),I=1,NP),
 { (NOP(4*(K-1)+M),K=1,NE),M=1,4),
 (IMAT(I),I=1,NE)

The following records are repeated for each load increment.

Record 1'. (1 word) Increment number, TINC.

Record 2'. (3x2xNP words) Nodal point displacements, repeated 3 times.

(TDIS(1,I),TDIS(2,I),I=1,NP),
 (TDIS(1,I),TDIS(2,I),I=1,NP),
 (TDIS(1,I),TDIS(2,I),I=1,NP)

Record 3'. (4xNE words) Average element stresses

((SPLT(I,J),J=1,NE),I=1,4)

Record 4'. (4xNE words) Average element strains

((SPLT(I,J),J=1,NE),I=1,4)

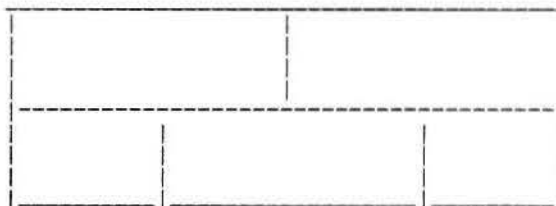
RUB.MOV

The records of this file are written to draw rubbleized groups of subelements to effectively display a failed region.

- Record 1. (2 words, E15.5, I5) Increment number, TT, and total number of parts, ICNT.
- Record 2. (4 words, 4I5) Number of "parts", NONE; number of nodal points, NUMNP; number of unfailed elements, NP2; number of entries in connectivity array, NCON.
- Record 3. (2 words, 2I5) Beginning element number, NP1; Ending element number, NP2.
- Record 4. (3xNUMNP words, 6E12.5) Nodal point coordinates
(CORD(I,1),CORD(I,2),0.0), I=1,NUMNP)
- Record 5. (4XNUMEL WORDS, 16I5) connectivity array
(IXAR(I),I=1,NCON)

The above records provide geometry data for the original mesh. The following records are written, for each and every failed or rubble element, to describe the rubbleization as a "rubble" region.

- Record 1'. (4 words, 4I5) The variables NONE, NJ, NPT AND NCON. These standard descriptors for the group of rubble subelements as used to describe a rubble region (see below). NONE is the number of "parts" for this group (i.e. 1), NJ is the number nodes (i.e. 12), NPT is the number of elements, (i.e. 5), and NCON is the number of entries in the connectivity (i.e. 20).



- Record 2'. (2 words, 2I5) The variables NP1 and NP2 which are the smallest and largest element numbers associated with the subelement group.
- Record 3'. (36 words, 6E12.5) the coordinates of the subelement group.
(RN(J),ZN(J),0.0), J=1,12)

SCRUBS.RST

This file is the restart file that is used to both restart from and write subsequent restart files. A file is automatically written after each converged increment. To restart a run, the following lines of SCRUBS.DAT, along with the proper SCRUBS.RST file, are all that are required.

1. Group 1, line 1.
2. Group 3, all lines
3. Group 6, lines 10,11,12,and 13.

Note: Group 6, line 10 (NINC) must be changed to the number of remaining load increments!*****

SCRUBS.LIS

This file contains input and output data formatted into a readable form. The input data is first written in its entirety, and includes a nodal mapping for the node numbers changed due to the slip plane addition.

After the input data is written, convergence information is output for each iteration. This information includes run times at various points in the program, and information on current loads, residuals, displacements, and plastic work. Convergence is reached when the normal of the residual sum ratio reaches the convergence percentage specified by the user. The flag NCHECK will be zero when convergence is attained.

The last portion of this file contains the output information. Output includes displacements and reactions for all boundary nodes, nodal displacements for all nodes if specified, and all stresses at each gauss point of any elements specified.

SCRUBS also has several checks for errors or incomplete solution which halt execution before the solution is obtained. For each of these an error message is output to the SCRUBS.LIS file. The following is a list of each of these stops, including a description and possible cause of the error.

NO CONVERGENCE ON INCREMENT I

Solution could not be reached in the number of iterations specified. This is usually due to too much plastic flow and can often be corrected by reducing the loads.

NEGATIVE DET STOPPED AT ELEMENT NO = I

X1,Y1= x , y

X2,Y2= x , y

X3,Y3= x , y

X4,Y4= x , y

This indicates a element is improperly deformed as shown



Usually caused by too large of deformations.

PROGRAM HALTED IN SET STIFFNESS SPACE EXCEEDED

Bandwidth is too high and stiffness dimensioning is exceeded, bandwidth must be decreased.

PROGRAM HALTED IN SOLVE

NEGATIVE OR ZERO DIAGONAL STIFFNESS

Stiffness matrix is not positive definite, error in material parameters or problem has rigid body modes.

ERROR-- FRACTURE SCPACING (DELTA) MUST BE POSITIVE & NONZERO

Fracture spacing has been input improperly

ERROR-- HALTED IN SLIPP, ERROR IN CONNECTIVITY

When slip plane is being added, improper slip node numbering leads to this error. Caused by improper mesh generation or adding of a slip plane along a free edge.

ERROR-- NODE I NOT FOUND IN CONNECTIVITY ARRAY

Program cannot find new node number of a node in the nodal mapping. Caused by error in mesh generation.

APPENDIX C

SCRUBS CODE LISTING

The following appendix contains the program listing of the computer code SCRUBS. This listing is included since SCRUBS was greatly updated in both logic and programming as this thesis was conducted. The subroutines will be found in alphabetical order. A summary of these subroutines with their descriptions can be found in appendix A.

•
•
• Subroutine BSUB:

• Conducts systematic back-substitution and solves for boundary
• displacements and reaction transformations.
•

• Written:

• Last modified: JUL 24 1984
•
•

• Called by: SCRUBS
•
•

SUBROUTINE BSUB

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

DIMENSION DIS(2,500)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

3 TPWORK

COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)

COMMON/BOUN/NBC(400),NFI(400),U(800),ANG(400),TRC(800),

1 US(400,2)

COMMON/LDS/R1(2000),RL(2000),RT(2000)

COMMON/SCR/SK(20000) ,R

EQUIVALENCE (SK(1),DIS(1,1))

REWIND 3

BACKSPACE 4

ND=ND-1

KJ=NDF*NP

DIS(NDF,NP)=R1(KJ)

R1(KJ)=0.

I=NDF

L=NP

NBUF=NBUF-2

IF(NNP EQ 1) BACKSPACE 4

GO TO 40

•
• Start systematic back substitution.
•

```

10      L=L-1
20      I=I-1
        NNP=SK(NBUF)
        IF(NNP.EQ.1) BACKSPACE 4
        NBUF=NBUF-I
        NSZ=SK(NBUF)
        IA=NBUF-NSZ
        NBUF=IA-3
        KJ=NDF*(L-1)+I
        GASH=R1(KJ)
        R1(KJ)=0.
        DO 30 J=1,NSZ
            LJ=L+(J+I-1)/NDF
            K=I+J-(LJ-L)*NDF
            GASH=GASH-SK(IA)*DIS(K,LJ)
30      IA=IA+1
        DIS(I,L)=GASH
40      IF(SK(NBUF+2).GT.0.) GO TO 50
        R1(KJ)=DIS(I,L)
        DIS(I,L)=U(ND)
        ND=ND-1
50      CONTINUE
        IF(I+L-2) 60,90,60
60      IF(NNP.NE.1) GO TO 70
        READ(4) NBUF,(SK(II),II=NSK,NBUF)
        BACKSPACE 4
70      IF(I.NE.1) GO TO 20
80      I=NDF+1
        GO TO 10
90      CONTINUE

```

•
• Boundary displacements and reaction transformation.
•

```

NOSH=0
DO 110 M=1,NB

```

```
MOSH=NOSH+1
NOSH=MOSH+1
N=NBC(M)
NASH=2*N
MASH=NASH-1
RX=R1(MASH)
RY=R1(NASH)
IF(ANG(M) EQ 0.) GOTO 100
GASH=ANG(M)*.017453292
CS=COS(GASH)
TN=TAN(GASH)
DXC=DIS(1,N)*CS
DYC=DIS(2,N)*CS
DIS(1,N)=DXC-DYC*TN
DIS(2,N)=DYC+DXC*TN
RXC=RX*CS
RYC=RY*CS
RX=RXC-RYC*TN
RY=RYC+RXC*TN
100 TRC(MOSH)=TRC(MOSH)+RX
TRC(NOSH)=TRC(NOSH)+RY
RT(MASH)=RT(MASH)+RX
RT(NASH)=RT(NASH)+RY
110 CONTINUE

RETURN
END
```

•

• Subroutine CONRUB:

• This subroutine checks for rubble/continuum interaction and
 • computes the hydrostatic force in the rubble if a
 • continuum element is in contact.

•

• Written:

• Last modified: JUL 19 1984

•

• Called by: RESIDUE

SUBROUTINE CONRUB(SIGRES,M)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,IPS,
 1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
 2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
 3 TPWORK,KNE
 COMMON/FG/ FLAG(1000),BULK(1000,2),MINE(10,50),NME(10),GAMMA
 COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
 1 ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
 COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)

SIGRES=.0001

•

• Computes area of rubble element.

K1=(M-1)*4*NSFR+1

K2=K1+NSFR

K3=K2+NSFR

K4=K3+NSFR

J1=NOP(K1)

J2=NOP(K2)

```

J3=NOP(K3)
J4=NOP(K4)
COE1=CORD(J2,1)-CORD(J4,1)
COE2=CORD(J3,2)-CORD(J1,2)
COE3=CORD(J3,1)-CORD(J1,1)
COE4=CORD(J4,2)-CORD(J2,2)
AREA= 5*(COE1*COE2+COE3*COE4)
IF(AREA.GE.BULK(M,2)) RETURN

```

•
• Computes hydrostatic compaction force since contact is established.
•

```

BSTN=(AREA-BULK(M,2))/BULK(M,2)
SIGRES=BSTN*ABS(ORT(NMAT,1))
WRITE(6,10) M,AREA,BULK(M,2),SIGRES
10  FORMAT(' M,AREA,BULK(M,2),SIGRES  !!!!!!!',I6,3E13.3)

RETURN
END

```

•
• Subroutine CONVG:
• Determines the percent convergence.
•
• Written:
• Last modified: JUL 19 1984
•

•
• Called by: SCRUBS
•

SUBROUTINE CONVG(NCHECK)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
DIMENSION RAB(2)

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,MALGO,PWORK,
3  TPWORK
COMMON/LDS/R1(2000),RL(2000),RT(2000)
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1  .NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
2  .STRS(4),STRN(4),R(3),RMAX(2),RTOT(3)
COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA

C      MASH=NCHECK=0
      MASH=0.0
      NCHECK=0.0
C      R(1)=R(2)=RMAX(1)=RMAX(2)=RTOT(1)=RTOT(2)=SUMD=SUMTD=0.
      R(1)=0.0
      R(2)=0.0
      RMAX(1)=0.0
      RMAX(2)=0.0
      RTOT(1)=0.0
      RTOT(2)=0.0
      SUMD=0.0
      SUMTD=0.0
      DO 20 I=1,NP
        DO 20 K=1,2
          MASH=MASH+1
          GISH=RT(MASH)
          GOSH=DIS(K,I)
          GESH=DISN(K,I)
          GASH=GISH-R1(MASH)
C      IF (1.GE.1.AND.1.LE.7) WRITE (6,10) I,K,RT(MASH),
C 1      R1(MASH),GASH
C      IF (1.GE.12.AND.1.LE.18) WRITE (6,10) I,K,RT(MASH),
C 1      R1(MASH),GASH
10     FORMAT(' *** I,K,RT(MASH),R1(MASH),GASH.**' 2I5,3E15.5)
          R1(MASH)=GASH
          R(K)=R(K)+GASH*GASH
          RTOT(K)=RTOT(K)+GISH*GISH
          SUMD=SUMD+GOSH*GOSH
          SUMTD=SUMTD+GESH*GESH
          GASH=ABS(GASH)
          IF(GASH.GT.RMAX(K))RMAX(K)=GASH

20     CONTINUE
      R(3)=R(1)+R(2)
      RTOT(3)=RTOT(1)+RTOT(2)
      DO 30 K=1,3

```

```

R(K)=SQRT(R(K))
RTOT(K)=SQRT(RTOT(K))
IF(RTOT(K).LT. 1E-10) RTOT(K)=1.
30 R(K)=100.*R(K)/RTOT(K)
SUMD=SQRT(SUMD)
SUMTD=SQRT(SUMTD)
IF(SUMTD.LT. 1E-20) SUMTD=1.
IF(SUMD.LT. 1E-20) SUMD=1.
SUM=100.*SUMD/SUMTD
IF(SUM.GT.CONFAC) NCHECK=1
WRITE(6,40)(RTOT(K),K=1,3),(R(I),I=1,3)
40 FORMAT(/5X,'X-NORM OF TOTAL LOADS =',E18.6/
1 5X,'Y-NORM OF TOTAL LOADS =',E18.6/
2 5X,'NORM OF SUM OF TOTAL LOADS =',E13.6/
3 5X,'X-NORM RESIDUAL RATIO =',E18.6/
4 5X,'Y-NORM RESIDUAL RATIO =',E18.6/
5 5X,'NORM OF RESIDUAL SUM RATIO =',E13.6,5X)
WRITE(6,50)SUMD,SUMTD,SUM,NCHECK
50 FORMAT(/5X,'NORM OF CURRENT INCREMENTED DISPLACEMENT =',E18.6/
1 5X,'NORM OF CURRENT TOTAL INCREMENTED DISPLACEMENT =',E12.6/
2 5X,'NORM OF DISPLACEMENT RATIO =',E18.6,5X,'NCHECK =',I2)
IF(IT.GT.1) GO TO 60
RAB(1)=RMAX(1)
RAB(2)=RMAX(2)
GO TO 80
60 DO 70 I=1,2
70 RMAX(I)=100.*RMAX(I)/RAB(I)
80 WRITE(6,90)(RMAX(K),K=1,2)
90 FORMAT(/5X,'MAXIMUM X-RESIDUAL =',E18.6/
1 5X,'MAXIMUM Y-RESIDUAL =',E18.6)
PRATIO=-100.
TPWORK=TPWORK+PWORK
IF(TPWORK.NE.0)PRATIO=100.*PWORK/TPWORK
WRITE(6,100)PWORK,TPWORK,PRATIO
100 FORMAT(/5X,'ITERATIVE PLASTIC WORK =',E18.6,2X,'INCREMENTAL'
1 ' PLASTIC WORK =',E18.6/
2 5X,'PERCENTAGE PLASTIC WORK =',F7.2)

RETURN
END

```

•

• Subroutine CORNN:

• This subroutine determines if node IX is a corner node.

•

• Written:

• Last modified: JUL 24 1984

•

• Called by: SLIPP

SUBROUTINE CORNN(NADD,NSFR,NP,IFLAG,IX)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)

1 .NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)

REAL CORN

J=0

10 J=J+1

IF(J.GT.4000) THEN

WRITE(6,20)IX

20 FORMAT(' ERROR -- NODE ',I3,' NOT FOUND IN '

1 'CONNECTIVITY ARRAY')

STOP

ELSE

IF(NOP(J).NE.IX)GOTO 10

CORN=FLOAT(J-1)/FLOAT(NSFR)

CORN=CORN-IFIX(CORN)

IF(CORN.NE.0.0)RETURN

NADD=NSFR

•

• If it is a corner node, IFLAG = 1.

IFLAG=1

RETURN
ENDIF

END

```

-----
-----
*
*      Subroutine DATALST:
*      Writes title, line numbers for files (presently not used).
*
*      Written:
*      Last modified: JUL 24 1984
*
-----
-----
*
*      Called by: SCRUBS
*
-----
-----

```

SUBROUTINE DATALST(IOIN,IOSC,IOPR)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/DLIST/ LN, LNCT, TITL(20)

C DIMENSION IA(8)

DIMENSION IA(20)

10 LN=LNCT

WRITE(IOPR,20) TITL

C20 FORMAT(6X,8A10//

20 FORMAT(6X,20A4//

1 14X,'10',8X,'20',8X,'30',8X,'40',8X,'50',8X,'60',8X,'70',8X,

2 '80',/,6X,'1234567890123456789012345678901234567890123456789012345678'

3 '90123456789012345678901234567890')

C30 READ(IOIN,40) IA

30 READ(IOIN,40,END=70) IA

C40 FORMAT(8A10)

40 FORMAT(20A4)

C IF(EOF(IOIN)) 70,50

50 WRITE(IOSC,40) IA

WRITE(IOPR,60) IA

C60 FORMAT(6X,8A10)

60 FORMAT(6X,20A4)

```

LN=LN-1
IF(LN.LE.0) GO TO 10
GO TO 30
70  ENDFILE IOSC
    REWIND IOSC

    RETURN
    END

```

```

-----
-----
*
*
*   Subroutine DROP:
*   Checks for elements about to fail and adjusts geometry for
*   failure.
*
*   Written:
*   Last modified: JUL 19 1984
*
-----
-----

```

```

*
*   Called by: RESIDUE
*
-----
-----

```

```

*
*   Subroutines called: RZBASEEXIT
*
-----
-----

```

SUBROUTINE DROP(NUMEL,NUMNP,AK)

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3  TPWORK,KNE
COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1  ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)
COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1  US(400,2)
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)

```

DIMENSION IBTMS(4),MSN(2,4)

•
•
• Check for elements ready to fall.
•
•

```

DO 200 I=1,NUMNP
  FLG=FLAG(I)
  IF(FLG.EQ.4.) GO TO 20
  IF(FLG.EQ.8.) GO TO 10
  GO TO 200
10  CONTINUE
  NN=2*I
  ZOLD=CORD(I,2)-TDIS(2,1)
  ZNEW=AK
  GO TO 100
20  CONTINUE

```

•
•
• Drop to znew.
•
•

ZOLD=CORD(I,2)-TDIS(2,1)

•
•
• Find bottom element numbers.
•
•

```

IBTMS(1)=0
IBTMS(2)=0
IBTMS(3)=0
IBTMS(4)=0
ICNT=0
L=NSFR*4
DO 40 N=1,NUMEL
  DO 30 K=1,4
    KK=(N-1)*NSFR*4+K
    IF(NOP(KK).NE.1) GO TO 30
    ICNT=ICNT+1

```

```

          IBTMS(ICNT)=N
30      CONTINUE
40      CONTINUE
        GO TO (50,60,70,70) ICNT

```

```

-----
*
*
*      ICNT=1, top corner node drop.
*
*
-----

```

```

50      IBOT=IBTMS(1)
        MX=IMAT(IBOT)
        IF(MX.GE.100) MX=MX-100
        AREA=BULK(IBOT,2)
        ALPH=ORT(MX,8)
        ANEW=AREA/ALPH
        IREF=(IBOT-1)*NSFR*4
        I1=NOP(IREF+1)
        I2=NOP(IREF+1+NSFR)
        I3=NOP(IREF+1+NSFR*2)
        I4=NOP(IREF+1+NSFR*3)
        CALL RZBASE(1,IBOT,I1,I2,I3,I4,R,Z,RLEN,ZBASE)
        ZLEN=ANEW/RLEN
        GO TO 90

```

```

-----
*
*
*      ICNT=2, side boundary element drop.
*
*
-----

```

```

60      CONTINUE
        DO 70 K=1,ICNT
          IBOT=IBTMS(K)
          IREF=(IBOT-1)*NSFR*4
          I1=NOP(IREF+1)
          I2=NOP(IREF+1+NSFR)
          I3=NOP(IREF+1+NSFR*2)
          I4=NOP(IREF+1+NSFR*3)
          ZAVG=(CORD(I1,2)+CORD(I2,2)+CORD(I3,2)+CORD(I4,2))*25
          ZAVG=ZAVG-.25*(TDIS(2,I1)+TDIS(2,I2)+TDIS(2,I3)+
1      TDIS(2,I4))
          MX=IMAT(IBOT)
          IF(MX.GE.100) MX=MX-100

```

```

      ALPH=ORT(MX,8)
      AREA=BULK(IBOT,2)
      IF(ZAVG.GT.ZOLD) GO TO 75
      MX=IMAT(IBOT)
      IF(MX.GE.100) MX=MX-100
      ALPH=ORT(MX,8)
      AREA=BULK(IBOT,2)
      ANEW=AREA/ALPH
      CALL RZBASE(1,IBOT,I1,I2,I3,I4,R,Z,RLEN,ZBASE)
      ZLEN=ANEW/RLEN
75    CONTINUE
      GO TO 90

-----
•
•
•      ICNT=4, interior node drop.
•
•
-----

70    CONTINUE
      ZBASET=0.0
      RLENT=0.0
      ALPH=0.0
      AREA=0.0
      ZDST=0.0
      DO 80 K=1,ICNT
         IBOT=IBTMS(K)
         IREF=(IBOT-1)*NSFR*4
         I1=NOP(IREF+1)
         I2=NOP(IREF+1+NSFR)
         I3=NOP(IREF+1+NSFR*2)
         I4=NOP(IREF+1+NSFR*3)
         ZAVG=(CORD(I1,2)+CORD(I2,2)+CORD(I3,2)+CORD(I4,2))* .25
         ZAVG=ZAVG-.25*(TDIS(2,I1)+TDIS(2,I2)+TDIS(2,I3)+
1      TDIS(2,I4))
         IF(ZAVG.GT.ZOLD) GO TO 80
         MX=IMAT(IBOT)
         IF(MX.GE.100) MX=MX-100
         ALPH=ALPH+ORT(MX,8)
         AREA=AREA+BULK(IBOT,2)
         CALL RZBASE(1,IBOT,I1,I2,I3,I4,R,Z,RLEN,ZBASE)
         RLENT=RLENT+RLEN
         ZBASET=ZBASET+ZBASE
         C1=CORD(I1,2)-TDIS(2,I1)
         C2=CORD(I2,2)-TDIS(2,I2)

```

```

      C3=CORD(I3,2)-TDIS(2,I3)
      C4=CORD(I4,2)-TDIS(2,I4)
      ZD1=ABS(C3-C1)
      ZD2=ABS(C2-C4)
      ZZZ=5*(ZD1+ZD2)
      ZDST=ZDST+ZZZ
80    CONTINUE
      ALPH=.5*ALPH
      ANEW=AREA/ALPH
C     ZLEN=ANEW/RLENT

```

```

*
*
*     ZLEN is recomputed here to circumvent errors when sloping elements
*     are used, ZLEN is computed as the average of the bulked vertical
*     distance between diagonals of one of the bottom elements.
*
*

```

```

      ZLEN=.5*ZDST
      ZLEN=ZLEN/ALPH
      ZBASE=5*ZBASET
90    CONTINUE
      ZNEW=ZBASE+ZLEN
      IF(ZNEW.GT.ZOLD) ZNEW=ZOLD
100   CONTINUE
      FLAG(I)=10.
      DRP=ZNEW-ZOLD
      CORD(I,2)=ZNEW
      TDIS(2,I)=DRP
      WRITE(6,110) I,ZNEW,ZOLD,DRP,AK,ZLEN,ZBASE,ANEW,RLEN,
1     ALPH,AREA
110   FORMAT(' I,ZNEW,ZOLD,DRP,AK,ZLEN,ZBASE,ANEW,RLEN,'
1     'ALPH,AREA',/I5,5E15.5/5E15.5)
      IF(NSFR.NE.2)GO TO 140
      DO 130 IE=1,4
        N={(IBTMS(IE)-1)*8
        DO 120 M=1,7,2
          MM=M+N
          IF(NOP(MM).NE.1)GO TO 120
          MSN(1,IE)=NOP(MM+1)
          IF(MM.EQ.7)MSN(2,IE)=NOP(MM-6)
120   CONTINUE
      IM=MSN(1,IE)
      II=MSN(2,IE)

```

```

      CORD(IM,2)=(CORD(1,2)+CORD(II,2))/2.0
130   TDIS(2,IM)=(TDIS(2,1)+TDIS(2,II))/2.0

-----
*
*   Until bulking is defined, fix nodes after fall.
*
-----

140   NBTOP=NB
      DO 160 KK=1,NB
          ICHECK=NBC(KK)
          IF(ICHECK.EQ.1) NFIX(KK)=11
          IF(ICHECK.EQ.1) GO TO 170
          IF(ICHECK.LE.1) GO TO 160
          NBTOP=NB+1
          NBCN=I
          NFIXN=11
          DO 150 JJ=KK,NBTOP
              NBCMOV=NBC(JJ)
              NFIXMOV=NFIX(JJ)
              NBC(JJ)=NBCN
              NFIX(JJ)=NFIXN
              NBCN=NBCMOV
              NFIXN=NFIXMOV
150   CONTINUE
          GO TO 170
160   CONTINUE
          NBTOP=NB+1
          NBC(NBTOP)=I
          NFIX(NBTOP)=11
170   NB=NBTOP
          IF(NBTOP.LE.400) GO TO 190
          WRITE(6,180)
180   FORMAT(' ***** NB EXCEEDS DIMENSIONED VALUE IN '
1     ' DROP*****')
          CALL EXIT
190   CONTINUE
200   CONTINUE

```

```

-----
*
*   Find bulked elements, set BULK(N,1)=2.0
*
-----

```



```

DO 210 I=1,NUMEL
  IREF=(I-1)*NSFR*4
  I1=NOP(IREF+1)
  I2=NOP(IREF+1+NSFR)
  I3=NOP(IREF+1+NSFR*2)
  I4=NOP(IREF+1+NSFR*3)
  SUM=FLAG(I1)+FLAG(I2)+FLAG(I3)+FLAG(I4)
  IF(SUM.NE.40.) GO TO 210
  BULK(I,1)=2.0
210 CONTINUE

```

```

RETURN
END

```

```

-----
-----
*
*
*   Subroutine FLAGE:
*   Makes impact node calculations and new node assignments.
*
*   Written:
*   Last modified: JUL 19 1984
*
-----
-----

```

```

*
*   Called by: SCRUBS
*
-----
-----

```

```

*
*   Subroutines called: NEWNOD   EXIT
*
-----
-----

```

SUBROUTINE FLAGE (NUMNP,NUMEL,NTAPE,IIF)

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3  TPWORK,KNE
COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)

```

```

1      ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)
      DIMENSION IBIE(100)

```

```

-----
•
•
•      Reads number of impact boundary nodes and writes it to
•      SCRUBS.LIS.
•
-----

```

```

      READ(NTAPE,10) NBIE
10     FORMAT(15)
      WRITE(6,20) NBIE
20     FORMAT(' NUMBER OF BOUNDARY IMPACT NODES IS',15)
      IF(NBIE.GT.100) GO TO 140
      IF(NBIE.EQ.0) GO TO 80
      NCARD=1+NBIE/16

```

```

-----
•
•
•      Writes impact node list after updating.
•
-----

```

```

      WRITE(6,30)
30     FORMAT(' IMPACT NODE LIST ')
      DO 80 K=1,NCARD
          KSTART=1+(K-1)*16
          KEND=KSTART+15
          READ(NTAPE,40) (IBIE(KK),KK=KSTART,KEND)
40     FORMAT(16I5)
          IF(IIF GE.0) GOTO 60
          DO 50 KK=KSTART,KEND
50     CALL NEWNOD(IBIE(KK))
60     WRITE(6,70)(IBIE(KK),KK=KSTART,KEND)
70     FORMAT(1X,16I5)
80     CONTINUE

```

```

-----
•
•
•      Makes impact node calculations.
•
-----

```

```

      DO 100 I=1,NUMNP

```

```

        ICOUNT=0
        FLAG(I)=0.0
        L=NSFR*4
        DO 90 J=1,NUMEL
            DO 90 K=1,L,NSFR
                KK=NSFR*4*(J-1)+K
                IF(NOP(KK).EQ.1) ICOUNT=ICOUNT+1
90      CONTINUE
            IF(ICOUNT.EQ.2) FLAG(I)=2.0
            IF(ICOUNT.EQ.1) FLAG(I)=3.0
100     CONTINUE
        IF(NSFR.NE.2) GOTO 120
        NOP8=NUMEL*8
        DO 110 I=2,NOP8,2
            MDS=NOPI
110     FLAG(MDS)=20.0
120     CONTINUE
        DO 130 I=1,NBIE
            IFIX=IBIE(I)
            IF(FLAG(IFIX).EQ.2.) FLAG(IFIX)=6.
            IF(FLAG(IFIX).EQ.3.) FLAG(IFIX)=7.
130     CONTINUE
        RETURN

-----
•
•      Error trap for excessive number of boundary nodes.
•
-----

140     WRITE(6,150) NBIE
150     FORMAT(' NBIE IS INPUT AS',I5,'BUT CANNOT BE LARGER
+      'THAN 100')
        CALL EXIT

        END

```

```

-----
-----
*
*      Subroutine GAUSSP: Initializes gaussian constants.
*
*      Written:
*      Last modified: MAR 19 1984
*
-----
-----
*
*      Called by: GDATA
*
-----
-----

```

```

SUBROUTINE GAUSSP(M,X,W)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
DIMENSION X(6),W(6)

GO TO (10,20,30,40,50,60),M
10  X(1)=0.
    W(1)=1.
    RETURN
20  X(1)=-0.577350269189626
    W(1)=1.
    GO TO 70
30  X(1)=-0.774596669241483
    X(2)=0.
    W(1)=0.555555555555556
    W(2)=0.888888888888889
    GO TO 70
40  X(1)=-0.861136311594053
    X(2)=-0.339981043584856
    W(1)=0.347854845137454
    W(2)=0.652145154862546
    GO TO 70
50  X(1)=-0.906179845938664
    X(2)=-0.538469310105683
    X(3)=0.
    W(1)=0.236926885056189
    W(2)=0.478628670499366
    W(3)=0.568888888888889
    GO TO 70

```

```

60      X(1)=-0.932469354203152
        X(2)=-0.661209386466265
        X(3)=-0.238619186083197
        W(1)=0.171324492379170
        W(2)=0.360761573048139
        W(3)=0.467913934572691
70      N=M/2
        DO 80 J=1,N
            NASH=M+1-J
            X(NASH)=-X(J)
80      W(NASH)=W(J)

        RETURN
        END

```

```

-----
-----
*
*      Subroutine GDATA:
*      Reads geometrical data, boundary conditions, and gaussian
*      integration data.
*
*      Written:
*      Last modified: JUL 24 1984
*
-----
-----

```

```

*
*      Called by SCRUBS
*
-----
-----

```

```

*
*      Subroutines called: SLIPPNODEXY
*      NEWNOD GAUSSP
*
-----
-----

```

SUBROUTINE GDATA

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

3 TPWORK

```

COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1  US(400,2)
COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1  NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
DIMENSION NOPE(12),DX(3),QUID(3),IB(12),NOPD(12)
REAL CORDDUM(1000,2)

```

```

-----
*
*
*   Reads number of slip planes (NNP), singular points (NSING),
*   boundary points (NNBB), summed reaction nodes (MPR),
*   indexing vector (IB), and writes to SCRUBS.LIS.
*
*
-----

```

```

5  READ(5,5) NNP,NSING,NNBB,MPR,(IB(I),I=1,12)
   FORMAT(16I5)
   IF(NB.GT.0.AND.NNBB.EQ.0) NNBB=NB
   WRITE(6,7)NNP,NSING,MPR,(IB(I),I=1,12)
7  FORMAT(1X,/,
1   '   NUMBER OF SLIP PLANES-----',I3//
2   '   NUMBER OF SINGULAR POINTS----',I3//
3   '   NO OF SUMMED REACTION NODES--',I3//
4   '   IB VECTOR-----',I2I4)
   IF(IB(1) NE 0) GO TO 40
   DO 10 I=1,12
10  IB(I)=NSFR
   IF(NSFR-2)40,30,20
C20  IB(5)=IB(6)=IB(11)=IB(12)=1
20  IB(5)=1
   IB(6)=1
   IB(11)=1
   IB(12)=1
   GO TO 40
C30  IB(4)=IB(8)=1
30  IB(4)=1
   IB(8)=1
40  CONTINUE
   DO 50 II=1,NP
   DO 50 JJ=1,NCORD
50  CORD(II,JJ) = 0.

```

•
•
• Reads nodal point data from QMESH9.DAT.
•
•

```

READ(9) (CORDDUM(N,1),CORDDUM(N,2),N=1,NP)
DO 60 N=1,NP
  CORD(N,1)=CORDDUM(N,1)
60  CORD(N,2)=CORDDUM(N,2)

```

•
•
• Reads element data from QMESH9.DAT.
•
•

```

IF (NCN.EQ.8) GO TO 70
READ(9)((NOP(NCN*(K-1)+M),M=1,NCN),IMAT(K)),K=1,NE)
GO TO 80
70  READ(9)((NOP(8*(K-1)+1),NOP(8*(K-1)+3),NOP(8*(K-1)+5),
•    NOP(8*(K-1)+7),NOP(8*(K-1)+2),NOP(8*(K-1)+4),
•    NOP(8*(K-1)+6),NOP(8*(K-1)+8),IMAT(K)),K=1,NE)

```

•
•
• Reads boundary data from QMESH9.DAT.
•
•

```

80  IF(NNBB.LE.0) GOTO 100
DO 90 I=1,NNBB
90  READ (5,95) NBC(I),NFIX(I), (US(I,J),J=1,NDF) ,ANG(I)
95  FORMAT(2I5,4F10.3)
100  CONTINUE

```

•
•
• Adds slip plane if needed.
•
•

```

IF(NNP.GT.0)CALL SLIPP

```

•
•
• Generates coordinates for intermediate nodes.
•
•

```
C      IF(NSFR.GT.1) CALL NODEXY
      IF(NSING.EQ.0) GOTO 140
      IF(NNP.GT.0) CALL NEWNOD(NSING)
```

•
• Singular point calculations.
•
•

```
      DO 130 K=1,NE
        DO 110 I=1,8
          ICK=NOP((K-1)*8+I)
          IF(ICK.EQ.NSING) GOTO 120
110      CONTINUE
          GOTO 130
120      IFP=I-2
          IF(IFP.EQ.-1) IFP=7
          IF(IFP.EQ.0) IFP=8
          NFAR=NOP((K-1)*8+IFP)
          XQP=0.25*CORD(NFAR,1)+0.75*CORD(NSING,1)
          ZQP=0.25*CORD(NFAR,2)+0.75*CORD(NSING,2)
          IQP=I-1
          IF(IQP.EQ.0) IQP=8
          NQP=NOP((K-1)*8+IQP)
          CORD(NQP,1)=XQP
          CORD(NQP,2)=ZQP
          IFP=I+2
          IF(IFP.EQ.9) IFP=1
          IF(IFP.EQ.10) IFP=2
          NFAR=NOP((K-1)*8+IFP)
          XQP=0.25*CORD(NFAR,1)+0.75*CORD(NSING,1)
          ZQP=0.25*CORD(NFAR,2)+0.75*CORD(NSING,2)
          IQP=I+1
          IF(IQP.EQ.9) IQP=8
          NQP=NOP((K-1)*8+IQP)
          CORD(NQP,1)=XQP
          CORD(NQP,2)=ZQP
130      CONTINUE
```



```

140     CONTINUE
      IF(NB.EQ.0) GO TO 180

```

•
•
•
•
•

Writes fixed boundary conditions to SCRUBS.LIS.

```

      WRITE(6,145)
145     FORMAT(' BOUNDARY CONDITIONS'/
1       ' NODE FIX X VALUE  Y VALUE  ANGLE')
      DO 150 I=1,NB
150     WRITE (6,95) NBC(I),NFIX(I), (US(I,J),J=1,NDF),ANG(I)
      IF(MPR.EQ.0) GO TO 180
      READ (5,5)(NOPR(I),I=1,MPR)
      IF(IIF.GE.0) GOTO 170
      DO 160 I=1,MPR
160     CALL NEWNOD(NOPR(I))
170     WRITE(6,5)(NOPR(I),I=1,MPR)
180     CONTINUE

```

•
•
•
•
•

Reads gaussian integration data and writes constants to SCRUBS.LIS.

```

      WRITE(6,190)
190     FORMAT(' GAUSSIAN INTEGRATION CONSTANTS')
      CALL GAUSSP(NGAUS,XG,CG)
      DO 210 I=1,NGAUS
      WRITE(6,200)XG(I),CG(I)
200     FORMAT(2F15.8)
210     CONTINUE
      CALL GAUSSP(MGAUS,XMG,CMG)
      DO 220 I=1,MGAUS
220     WRITE(6,200)XMG(I),CMG(I)

      RETURN
      END

```

•
• Subroutine INVAR:
• Calculates invariants and sets the yield stress accordingly.

•
• Written:
• Last modified: JUL 24 1984
•

•
• Called by: RESIDUESTIFM
•

SUBROUTINE INVAR (SSS)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

3 TPWORK

COMMON/INV/SMEAN,STJ2,STJ3,SIGMA,PHI,APNI,STN,CPhi,YIELD

COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST,

1 TABSTN(10),TABSTR(10,15)

DIMENSION SSS(4)

•
• Calculates invariants.
•

X=SSS(1)

Y=SSS(2)

Z=SSS(4)

XY=SSS(3)

SMEAN=(X+Y+Z)/3.

C SSS(1)=X-X-SMEAN

TEMP=X-SMEAN

X=TEMP

SSS(1)=TEMP

C SSS(2)=Y=Y-SMEAN

```

TEMP=Y-SMEAN
Y=TEMP
SSS(2)=TEMP
C   SSS(4)=Z-Z-SMEAN
TEMP=Z-SMEAN
Z=TEMP
SSS(4)=TEMP
STJ2=XY*XY+.5*(X*X+Y*Y+Z*Z)
IF(STJ2.EQ.0.0) STJ2=.00001
STJ3=Z*(Z-Z-STJ2)
SIGMA=SQRT(STJ2)
STN=-2.5980762113*STJ3/(STJ2*SIGMA)
IF(STN.LT.1.0.AND.STN.GT.-1.0) GO TO 10
IF(STN.LE.-.9999) STN=-.9999
IF(STN.GE..9999) STN=.9999
10  CONTINUE
PHI=ASIN(STN)/3.
CPHI=COS(PHI)
CON=1.73205080756888
MYIELD=NYIELD
IF(NYIELD.GT.100)MYIELD=NYIELD-100
GO TO (20,30,40,50),MYIELD

```

```

-----
•
•   Von Mises yield condition.
•
•
-----

```

```

20  YIELD=CON*SIGMA
RETURN

```

```

-----
•
•   Tresca yield condition.
•
•
-----

```

```

30  YIELD=2.*SIGMA*CPHI
RETURN

```

```

-----
•
•   Drucker yield condition.
•
•

```

```

-----
40      YIELD=3.*ALPHA*SMEAN+SIGMA
      RETURN

```

```

-----
•
•      Beltrami yield condition.
•
-----

```

```

50      YIELD=SQRT((3.+6.*PR)*SMEAN*SMEAN+2.*(1.-PR)*STJ2)

      RETURN
      END

```

```

-----
•
•      Subroutine JOINTM:
•      Calculates incremental stresses and residuals.
•

```

```

•      Written:
•      Last modified: JUL 19 1984
•
-----

```

```

-----
•
•      Called by: RESIDUE
•
-----

```

```

-----
•
•      Subroutines called: ROTATE
•
-----

```

```

SUBROUTINE JOINTM(L,STRN,STRS,MOSH,YIELD,PSTRN)

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

```

DOUBLE PRECISION K,MU,LAMB

```

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

```

```

1      NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

```

```

2      NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

```

```

3      TPWORK
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1      ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
2      ,SIG(4),PSIG(4),DSIG(4),EP(4),NG(16),NGSTP(16)
      DIMENSION STRN(4),STRS(4),T(4),TT(4),E(4)

```

•
•
• Assigns material parameters.
•
•

```

      K=ORT(L,1)
      G=ORT(L,2)
CO=ORT(L,3)
MU=ORT(L,4)
      TH=ORT(L,5)
FAIL=ORT(L,7)
DEL=ORT(L,9)
GS=ORT(L,10)

```

•
•
• Checks for delta and Gs equal to zero.
•
•

```

      IF(GS EQ 0.0)GS=1.0
      IF(DEL.GT.0) GOTO 20
      WRITE(6,10)
10      FORMAT(/,' ERROR-- FRACTURE SPACING (DELTA) MUST BE'
&      ' POSITIVE & NONZERO')
      STOP

```

•
•
• Rotates incremental strain vector to local system.
•

```

•      z or y/ / /
•      ^ / / / <- crack array
•      | / / /
•      | / / /
•      | / / /
•      | / / /
•      | / / /

```

```

•      | / /
•      | / / <- angle theta
•      -----> r or x
•
•      Coordinate system for crack array
•
-----
20      CONTINUE
      STRN(3)=STRN(3)/2.
      CALL ROTATE(STRN,E,TH)
•
-----
•
•      Gets total stress vector and rotates.
•
-----

      DO 30 I=1,4
      STRS(I)=TSTS(I,MOSH)
30      CONTINUE
      CALL ROTATE(STRS,TT,TH)
•
-----
•
•      Solves for incremental stresses.
•
-----

      TRTDOT=(K-2.*G/3.)*(E(1)+E(2)+E(4))
      T(1)=2.*G*E(1)+TRTDOT
      T(2)=2.*G*E(2)+TRTDOT
C      T1TOT=TT(1)+T(1)
      T2TOT=TSTS(2,MOSH)+T(2)
•
-----
•
•      Tests for tensile failure of crack.
•      However, for this application allows tensile strength.
•
-----

C      FAIL=0.0
C      IF(T1TOT.GT.FAIL) THEN
C      WRITE(6,*) FAILURE STRESS= ,T1TOT

```

```

C      T(1)=0.0
C      ENDIF
C      FAIL=0.0
C      IF(T2TOT.GT.FAIL) THEN
C          WRITE(6,*) ' FAILURE STRESS= ',T2TOT
C          TSTS(2,MOSH)=FAIL
C          T(2)=0.0
C      ENDIF
T(3)=2.*G*E(3)/(1.+G/DEL/GS)
T(4)=2.*G*E(4)+TRTDOT

```

•

• Checks for plastic slip.

•

```

C      F=DABS(TT(3)+T(3))+MU*(TT(1)+T(1))-CO
      F=DABS(TT(3)+T(3))+MU*(TT(2)+T(2))-CO
IF(F.LE.0) GOTO 50
XX=(2.*G/(1.+G/DEL/GS))
Y=XX*E(3)+TT(3)

```

•

• Solves for plastic strain.

•

```

C      IF (Y.LE.0) LAMB=E(3)+(CO-MU*(TT(1)+T(1))+TT(3))/XX
C      IF (Y.GT.0) LAMB=E(3)-(CO-MU*(TT(1)+T(1))-TT(3))/XX
      IF (Y.LE.0) LAMB=E(3)+(CO-MU*(TT(2)+T(2))+TT(3))/XX
      IF (Y.GT.0) LAMB=E(3)-(CO-MU*(TT(2)+T(2))-TT(3))/XX

```

•

• Determines shear stress for plastic condition.

•

```

40      CONTINUE
      ELAS=E(3)-LAMB
      T(3)=2.*G*ELAS/(1.+G/DEL/GS)
      EPSTN(5,MOSH)=EPSTN(5,MOSH)+LAMB

```

```

50      CONTINUE
C      YIELD=DABS(TT(1)+T(1))*MU
      YIELD=DABS(TT(2)+T(2))*MU

```

```

-----
*
*      Rotates incremental stress matrix to global system.
*
-----

```

```

CALL ROTATE(T,STRS,-TH)
STRN(3)=STRN(3)*2.

```

```

RETURN
END

```

```

-----
*
*      Subroutine LDATA: Reads load control data.
*

```

```

*      Written:
*      Last modified: JUL 14 1984
*

```

```

-----
*
*      Called by: SCRUBS
*

```

```

-----
*
*      Subroutines called: SFRAUX
*      NEWNOD
*

```

```

-----
SUBROUTINE LDATA(IIF)

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

```

DIMENSION R(3)

```

```

DIMENSION NOPL(4),PS(4,2),PQ(2),DJ(2),DENS(10)

```

```

COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA

```

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

```



```

1   NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2   NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO, PWORK,
3   TPWORK
   COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
   COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
   COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST
1   ,TABSTN(10,15),TABSTR(10,15)
   COMMON/LDS/R1(2000),RL(2000),RT(2000)
   COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),
1   NOFE(12),DISE(2,12),RP,RPB,XYP(3)
   COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1   ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
   COMMON/GRAV/GRAV

```

•
•
• Reads and writes load control data.
•
•

```

   READ(5,10)TITLE
10  FORMAT(20A4)
   READ(5,20)NSTRS,NRE,NRG,NRPRS,NRT,NRC
20  FORMAT(8I5)
   WRITE(6,30)TITLE,ILD,NSTRS
30  FORMAT(1X,20A4,5X, 'LOAD CASE=',I3,I5)
   WRITE(6,40)NRE,NRG ,NRPRS,NRT,NRC
40  FORMAT(' NRE=',I3,2X,'NRG=',I2,2X,'NRPRS=',I2,2X,'NRT=',
1   I2,2X,'NRC=',I2)
   IF(NRE EQ 0) GO TO 80

```

•
•
• Reads external loads at nodes.
•
•

```

   WRITE(6,50)
50  FORMAT(' LOADS FROM CARDS ')
   DO 70 I=1,NRE
       READ(5,60) NQ,(R(K),K=1,NDF)
60  FORMAT(I10,3F10.3)

```

•

• Changes node no. to new node no. if slip plane has been added.

```

•
•
•-----
      IF(IIF.LT.0)CALL NEWNOD(NQ)
      WRITE(6,60) NQ,(R(K),K=1,NDF)
      DO 70 K=1,NDF
        IC=(NQ-1)*NDF+K
        RL(IC)=RL(IC)+R(K)
70    CONTINUE

```

• Reads gravity, centrifugal and density data.

```

•-----
•
•
•
80    CONTINUE
      IF((NRC+NRG).EQ.0) GOTO 230
      READ(5,90)THETA,GRAV,ANGVEL
90    FORMAT(8E10.3)
      WRITE(6,100)THETA,GRAV,ANGVEL
100   FORMAT(' ANGLE=' ,F7.2,' GRAVITY CONSTANT=' ,E10.4,' ANGULAR VELOC'
1     'ITY=' ,F10.4)
      READ (5,90)(DENS(I),I=1,NMAT)
      WRITE(6,90)(DENS(I),I=1,NMAT)
      DENSAB=0.0
      DO 110 IA=1,NMAT
110   DENSAB=DENSAB+DENS(IA)
      GAMMA=DENSAB*GRAV/NMAT
      GUSH=0.
      IF(NRC.EQ.0) GO TO 120
      GUSH=ANGVEL*ANGVEL/GRAV
120   THETA=THETA/57 295779514
C     WRITE(6,130)
130   FORMAT(' DENSITY CONSTANTS FOR VARIOUS ELEMENTS')
      DO 220 M=1,NE
        L=IMAT(M)
        IF(L.GT.NMAT)L=L-100
        THICK=ORT(L,6)
        DENSE=DENS(L)
C     PQ(1)=PQ(2)=0.
        PQ(1)=0.
        PQ(2)=0.
        IF(DENSE.EQ.0.)GO TO 220

```

```

      IF(NRC.NE.0)GOTO 150
C      DO 140 IJ=1,NMAT
C140     DENS(IJ)=DENS(IJ)*GRAV
150     IF(NRG.EQ.0) GO TO 170
      PQ(1)=DENSE*SIN(THETA)*GRAV
      PQ(2)=-DENSE*COS(THETA)*GRAV
C      WRITE(6,160)M,L,PQ(1),PQ(2),DET
160     FORMAT(2I10,2F15.5,3X,D12.3)
170     NASH=NCN*(M-1)
      DO 190 K=1,NCN
      MASH=NOP(NASH+K)
      DO 180 I=1,NCORD
180     XYE(I,K)=CORD(MASH,I)
190     NOPE(K)=MASH
      DO 210 IGAUS=1,NGAUS
      DO 210 JGAUS=1,NGAUS
      G=XG(IGAUS)
      H=XG(JGAUS)
      CALL SFR(G,H)
      CALL AUX(M)
      DV=DET*CG(IGAUS)*CG(JGAUS)
      IF(NPP.EQ.2)DV=DV*RP*6.28318530718
      IF(NCORD.EQ.3)DV=DV*XYP(3)
      IF(THICK.NE.0)DV=DV*THICK
      PQ(1)=PQ(1)+DENSE*GUSH*XYP(1)
      DO 200 I=1,NCN
      IC=(NOPE(I)-1)*2
      DO 200 K=1,2
      IC=IC+1
200     RL(IC)=RL(IC)+PQ(K)*P(I)*DV
210     CONTINUE
220     CONTINUE
230     CONTINUE
      IF(NRPRS.EQ.0) GO TO 330
      READ(5,20)LNE
      WRITE(6,20)LNE
      NLN=NSFR+1

```

•
•
• Reads pressure data.
•
•

```

DO 320 LNEW=1,LNE

```

```
READ (5,20)(NOPL(J),J=1,NLN)
```

```
-----
*
* Changes node no. to new node no. if slip plane has been added.
*
*
-----
```

```
DO 240 J=1,NLN
240 IF(IIF.LT.0)CALL NEWNOD(NOPL(J))
WRITE(6,20)(NOPL(J),J=1,NLN)
```

```
-----
*
* Reads pressure load.
*
*
-----
```

```
READ (5,250)((PS(I,J),I=1,NLN),J=1,NDF)
250 FORMAT(8F10.3)
IF(PS(3,1).NE.0.)GO TO 270
PRT=PS(2,1)
DO 260 I=1,NLN
PS(I,1)=PS(1,1)
260 PS(I,2)=PRT
270 WRITE(6,250)((PS(I,J),I=1,NLN),J=1,NDF)
H=-1.
DO 280 J=1,NLN
MASH=NOPL(J)
DO 280 K=1,NCORD
XYE(K,J)=CORD(MASH,K)
280 CONTINUE
DO 310 IGAUS=1,NGAUS
G=XG(IGAUS)
CALL SFR(G,H)
DO 300 I=1,NCORD
C GASH=GOSH=GISH=0.
GASH=0
GOSH=0
GISH=0
DO 290 K=1,NLN
GASH=GASH+XYE(I,K)*P(K)
IF(I.GT.2) GO TO 290
GOSH=GOSH+PS(K,I)*P(K)
GISH=GISH+XYE(I,K)*DEL(1,K)
```

```

290          CONTINUE
           XYP(1)=GASH
           IF(I GT 2) GO TO 300
           PQ(1)=GOSH
           DJ(1)=GISH
300          CONTINUE
           DV=CG(IGAUS)
           IF(NCORD.EQ.3)DV=DV*XYP(3)
           IF(NPP.EQ.2)DV=6.2831853072*DV*XYP(1)
           DV1=DV*DJ(1)
           DV2=DV*DJ(2)
           AY=DV1*PQ(1)+DV2*PQ(2)
           AX=DV1*PQ(2)-DV2*PQ(1)
           DO 310 I=1,NLN
             NASH=2*NOPL(I)
             MASH=NASH-1
             RL(MASH)=RL(MASH)+P(I)*AX
             RL(NASH)=RL(NASH)+P(I)*AY
310          CONTINUE
320          CONTINUE
330          CONTINUE
C           WRITE(6,340)
340          FORMAT(' TOTAL NON-ZERO LOADS'/2X,
1           NODE X VALUE Y VALUE')
           I=1
350          IF(RL(I).EQ.0.) GO TO 360
           K=(10*I/NDF+9) / 10
           J=(K-1)*NDF+1
           JND=J+NDF-1
C           WRITE(6,60) K, (RL(I), I=J,JND)
           I=JND
360          CONTINUE
           I=I+1
           IF(I.LE.NSZF) GO TO 350

           RETURN
           END

```

•
• Subroutine LINEAR:
• Helps to update stress and strain.

•
• Written:
• Last modified: JUL 19 1984
•

•
• Called by: RESIDUE
•

SUBROUTINE LINEAR(STRN,STRS,W)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD

COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),

1 NOPE(12),DISE(2,12),RP,RPB,XP(3)

COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST,

1 TABSTN(10),TABSTR(10,15)

DIMENSION STRN(4),STRS(4)

DO 20 I = 1,2

DO 20 J = 1,2

GASH=0

DO 10 K = 1,NCN

10 GASH=GASH+DISE(I,K)*DI(J,K)

20 DIJ(I,J)=GASH

•
• Calculate strain at a point.
•

STRN(1)=DIJ(1,1)

STRN(2)=DIJ(2,2)

STRN(3)=(DIJ(1,2)+DIJ(2,1))

STRN(4)=0.

Calculate rotation, W.

```

W=5*(DIJ(2,1)-DIJ(1,2))
IF(NPP.NE.2) GO TO 40
GISH=0.
DO 30 K=1,NCN
30  GISH=GISH+DISE(1,K)*P(K)
   IF(RP.EQ.0.) RP=.000001
   STRN(4)=GISH/RP
40  IF(NPP.EQ.1)STRN(4)=-PPR*(STRN(1)+STRN(2))
   GASH=CLAME*(STRN(1)+STRN(2)+STRN(4))
   DO 50 I=1,4
50  STRS(I)=GM*STRN(I)+GASH
   STRS(3)=(STRS(3)-GASH)*.5

RETURN
END

```

Subroutine MAKEEL.

This subroutine adds a new element to the connectivity containing
corner nodes I and K.

Written:

Last modified: JUL 19 1984

Called by: SLIPP

SUBROUTINE MAKEEL(MMM,I,K,IANG)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

```

2      NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3      TPWORK,KNE
      COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1      ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)

      JSTART=I
      IF(IANG.EQ.2)GOTO 10
      IF(CORD(I,2).LT.CORD(K,2))JSTART=K
      GOTO 20
10     IF(CORD(I,1).GT.CORD(K,1))JSTART=K
20     CONTINUE
      L=NSFR*4
      DO 30 M=1,L,NSFR
MM=MMM+M
      IF(NOP(MM).NE.JSTART)GOTO 30
      JSTRT=MM
      IFLAG=M
30     CONTINUE
      NEW=NE*L+1
      NOP(NEW)=NOP(JSTRT)
      DO 40 N=1,NSFR
NEW=NEW+1
      IF(IFLAG.EQ.1)JSTRT=JSTRT+L
      JSTRT=JSTRT-1
40     NOP(NEW)=NOP(JSTRT)
      DO 50 N=1,NSFR
NOP(NEW+1)=NOP(NEW)+1
50     NEW=NEW+1
      IX=NSFR-1
      DO 60 N=1,IX
60     NOP(NEW+N)=NOP(NEW-NSFR-N)+1
      NEW=NEW+IX
      DO 70 N=NSFR,1,-1
NEW=NEW+1
70     NOP(NEW)=JSTART+N
      NE=NE+1
      IMAT(NE)=NMAT-NL

      RETURN
      END

```

•
• Subroutine MOD:
• Forms D matrix for different stress/strain cases.

•
• Written:
• Last modified: JUL 14 1984
•

•
• Called by: RESIDUE STIFM
•

SUBROUTINE MOD(L,M)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
& NCORD
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST

YM=ORT(L,1)
PR=ORT(L,2)
YST=ORT(L,3)
HM=ORT(L,4)
ALPHA=ORT(L,5)
DELTA=ORT(L,9)
GS=ORT(L,10)

•
• Allows for bulk properties.
•

IF(YM.LT.0.0) GO TO 30
PPR=PR/(1.-PR)
GM=YM/(1.+PR)

•

• Note!!!!!!***** GM=2*SHEAR MODULUS *****

•

•-----

```

      CLAME=PR*GM/(1.-2.*PR)
      IF(M.EQ.1)RETURN
      D(3,3)=GM* 5
C      D(1,3)=D(3,1)=D(2,3)=D(3,2)=0.
      D(1,3)=0.0
      D(3,1)=0.0
      D(2,3)=0.0
      D(3,2)=0.0
      IF(NPP.EQ. 1) GO TO 10
      YM=YM/(1.-PR*PR)
      PR=PR/(1.-PR)
10     CONTINUE

```

•-----

•

• For plane stress/strain cases.

•

•-----

```

      CD=YM/(1.-PR*PR)
      CDU=CD*PR
C      D(1,1)=D(2,2)=CD
      D(1,1)=CD
      D(2,2)=CD
C      D(1,2)=D(2,1)=CDU
      D(1,2)=CDU
      D(2,1)=CDU
      IF(NPP.NE.2) GO TO 20

```

•-----

•

• D matrix for axisymmetric case.

•

•-----

```

      D(4,4)=CD
C      D(1,4)=D(4,1)=D(2,4)=D(4,2)=CDU
      D(1,4)=CDU
      D(4,1)=CDU
      D(2,4)=CDU
      D(4,2)=CDU

```

```

C      D(3,4)=D(4,3)=0.
      D(3,4)=0.
      D(4,3)=0.
20     CONTINUE
      PR=ORT(L,2)
      RETURN
30     CONTINUE
      BM=-YM
      GM=ABS(PR)*2.

```

```

-----
*
*
*      Note!!!!!!***** GM=2*SHEAR MODULUS *****
*
*
-----

```

```

      CLAME=BM-GM/3.
      PR= 5
      PPR=PR/(1.-PR)
      IF(M EQ 1) RETURN
      D(1,1)=CLAME+GM
      D(2,2)=CLAME+GM
      D(3,3)=GM/2.
      IF(ORT(L,2).LT.0) D(3,3)=D(3,3)/(1.+GM/2./DELTA/GS)
      D(1,2)=CLAME
      D(1,3)=0.0
      D(2,1)=CLAME
      D(2,3)=0.0
      D(3,1)=0.0
      D(3,2)=0.0
      IF(NPP.NE.2) GO TO 40
      D(4,4)=CLAME+GM
      D(1,4)=CLAME
      D(2,4)=CLAME
      D(3,4)=0.0
      D(4,1)=CLAME
      D(4,2)=CLAME
      D(4,3)=0.0
40     RETURN
      END

```

```

-----
-----
*
* Subroutine NEWNOD:
* This subroutine finds the new node number of node N.
*
* Written:
* Last modified: JUL 19 1984
*
-----
-----

```

```

*
* Called by: FLAGEGDATAALDATA
* SLIPP
*
-----
-----

```

SUBROUTINE NEWNOD(N)

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3 TPWORK,KNE
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2),
1 NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)
COMMON/SLP/DCOR(1000,2)

K=0
DO 10 I=1,NP
10 IF(DCOR(N,1) EQ CORD(I,1).AND.DCOR(N,2) EQ CORD(I,2)) K=I
N=K
IF(K NE 0) RETURN
WRITE(6,20) N
20 FORMAT(' ***** ERROR IN MAPPING: NO NEW NODE FOUND FOR OLD '
+ 'NODE ',I3,' *****')

STOP
END

```

```

-----
-----
*
*
*   Subroutine NFLOW:
*   Calculates the amount of yield which results.
*
*   Written:
*   Last modified: MAR 19 1984
*
-----
-----

```

```

-----
-----
*
*   Called by: RESIDUESTIFM
*
-----
-----

```

```

SUBROUTINE NFLOW(SSS)

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3  TPWORK
COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST,
1  TABSTN(10,15),TABSTR(10,15)
COMMON/INV/SMEAN,STJ2,STJ3,SIGMA,PHI,APHI,STN,CPHI,YIELD
COMMON/FLW/ABETA,AA(4),DD(4)
DIMENSION SSS(4),A1(4),A2(4),A3(4)

```

```

-----
*
*   Initializes vector A1.
*
-----

```

```

C   A1(1)=A1(2)=A1(4)=1.
    A1(1)=1.
    A1(2)=1.
    A1(4)=1.
    A1(3)=0.

```

```

-----
*
*   Calculates vector A2.

```

```

*
-----
      SSS(3)=2.*SSS(3)
      GASH=2.*SIGMA
      DO 10 I=1,4
10      A2(I)=SSS(I)/GASH

```

```

*
*
*      Calculates vector A3.
*
-----

```

```

      CON=ST J2/3
      A3(1)=CON+SSS(2)*SSS(4)
      A3(2)=CON+SSS(1)*SSS(4)
      A3(3)=-SSS(4)*SSS(3)
      A3(4)=CON+ SSS(1)*SSS(2)- 25*SSS(3)*SSS(3)
      CON=1.73205080756888
      APHI=PHI*57.29577951308232
      MYIELD=NYIELD
      IF(NYIELD.GT.100)MYIELD=NYIELD-100
      GO TO (20,30,50,60),MYIELD

```

```

*
*
*      Von Mises yield criteria.
*
-----

```

```

20      C1=0.
      C2=CON
      C3=0.
      GO TO 70

```

```

*
*
*      Tresca yield criteria.
*
-----

```

```

30      C1=0.
      AAPHI=ABS(APHI)
      IF(AAPHI.LT.29.0) GO TO 40

```

```

C2=CON
C3=0.
GO TO 70
40  SN=SIN(PHI)
    CTS=SQRT(1.-STN*STN)
    C2=2.*(CPHI+SN*STN/CTS)
    C3=CON*SN/(STJ2*CTS)
    GO TO 70

```

•
• Drucker yield criteria.
•

```

50  C1=ALPHA
    C2=1.
    C3=0.
    GO TO 70

```

•
• Beltrami yield criteria.
•

```

60  C1=(1.+2.*PR)*SMEAN/YIELD
    C2=(1.-PR)*GASH/YIELD
    C3=0
70  CONTINUE
    DO 80 I=1,4
80  AA(I)=C1*A1(I)+C2*A2(I)+C3*A3(I)
    IF(NPP.NE.1) GO TO 90
    CON=AA(4)
    AA(4)=-PPR*(AA(1)+AA(2))
90  GASH=CLAME*(AA(1)+AA(2)+AA(4))
    GISH=HM
    DO 100 I=1,4
        DD(I)=GM*AA(I)+GASH
        IF(I.EQ.3)DD(I)=.5*AA(I)*GM
100  GISH=GISH+AA(I)*DD(I)
    ABETA=1./GISH
    IF(NPP.EQ.1)AA(4)=CON

    RETURN

```

END

```

-----
-----
*
* Subroutine NODEXY:
* Sets x,y values of new intermediate coordinates of quadratic or
* cubic elements.
*
* Written:
* Last modified MAR 19 1984
*
-----
-----

```

```

-----
-----
*
* Called by: GDATASLIPP
*
-----
-----

```

SUBROUTINE NODEXY(ISTART)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1 NCORD
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2),
1 NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)

ANSFR=NSFR
NQ=NSFR-1
DO 30 N=ISTART,NE
  NASH=NCN*(N-1)
  DO 30 NLV=1,NCN,NSFR
    NV1=NOP(NASH+NLV)
    NL1=NLV+NSFR
    IF(NL1.GT.NCN) NL1=1
    NV2=NOP(NASH+NL1)
    DO 20 I=1,NQ
      M=NLV+I
      J=NOP(NASH+M)
      GISH=I
      GOSH=ANSFR-GISH
      DO 10 K=1,2
10 CORD(J,K)=(GOSH*CORD(NV1,K)+CORD(NV2,K)*GISH)/ANSFR
20 CONTINUE

```


30 CONTINUE

RETURN
END

```

-----
-----
*
*
* Subroutine OUTPUT:
* Writes displacements, reactions, stresses and strains to SCRUBS.LIS
*
* Written:
* Last modified: JUL 19 1984
*
-----
-----
*
* Called by: SCRUBS
*
-----
-----

```

SUBROUTINE OUTPUT

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/ELPRNT/ NHELP,NPEL(50),NONODE
DIMENSION SAVG(6)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3 TPWORK
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/BOUN/NBC(400),NPIX(400),U(800),ANG(400),TRC(800),
1 US(400,2)
COMMON/LDS/R1(2000),RL(2000),RT(2000)
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2),
1 NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)

MOUT=NOUT(INC)
IF(MOUT.LT.100)GO TO 10
M=MOUT/100
MOUT=MOUT-M*100
IF(IT.EQ.1)MOUT=M
10 IF(MOUT.EQ.0)RETURN
IF(NONODE.GT.0) GO TO 50

```

Writes x and y displacements.

```

WRITE(6,20)
20  FORMAT(' DISPLACEMENTS',5X,'X',15X,'Y',13X,'TOTAL X',11X,
1    'TOTAL Y')
    DO 30 N=1,NP
30   WRITE(6,40)N,(DIS(I,N),I=1,NDF),(TDIS(I,N),I=1,NDF)
40   FORMAT(I10,6E16.6)
50   CONTINUE
    IF(MOUT.EQ.1)RETURN

```

Writes node numbers and corresponding displacements and reactions.

```

WRITE(6,60)
60  FORMAT(6X,'NODE NO',10X,'DISPLACEMENTS',16X,'REACTIONS')
    NASH=0
    DO 70 I=1,NB
        N=NBC(I)
        MASH=NASH+1
        NASH=MASH+1
70  WRITE(6,80)N,(TDIS(J,N),J=1,2),(TRC(J),J=MASH,NASH)
80  FORMAT(I10,6E16.6)
    IF(MPR.EQ.0)GO TO 130
C   SUMX=SUMY=0.0
    SUMX=0.0
    SUMY=0.0
    DO 110 I=1,MPR
        NASH=NOPR(I)
        DO 90 J=1,NB
            N=NBC(J)
            IF(N.EQ.NASH) GO TO 100
90   CONTINUE
        GO TO 110
100  MASH=2*J
        SUMX=SUMX+TRC(MASH-1)
        SUMY=SUMY+TRC(MASH)

```

110 CONTINUE

```

-----
*
*   Writes total reactions at specified points.
*
-----

```

```

WRITE(6,120)SUMX,SUMY
120  FORMAT(' TOTAL REACTIONS AT SPECIFIED POINTS  ALONG X= ',
1     E16.6,' ALONG Y= ', E16.6)
130  IF(MOUT.EQ.2)RETURN

```

```

-----
*
*   Writes stresses at sampling points.
*
-----

```

```

WRITE(6,140)
140  FORMAT(' STRESSES AT SAMPLING POINTS '/
1     ' GAUSS P',1X,'SIG X',6X,'SIG Y',6X,'TAU XY',6X,' SIG Z',6X,
2     ' YIELD',6X,'PL. STRN')
MCN=NSTORE/NE
DO 290 M=1,NE
  IF(NELP.LT.0) GO TO 160
  DO 150 K=1,NELP
    IF(M.EQ.NPEL(K)) GO TO 160
150  CONTINUE
    GO TO 290
160  CONTINUE
    WRITE(6,170) M
170  FORMAT(' EL NO ',I3)
    K=MCN*(M-1)
    EPS=0.
    DO 190 L=1,MCN
      J=K+L
      WRITE(6,180)L,(TSTS(I,J),I=1,5),EPSTN(5,J)
180  FORMAT(I2,6(1PE12.3))
      EPS=EPS+EPSTN(5,J)
190  CONTINUE
    XMCN=MCN
    EPS=EPS/XMCN
    DO 200 I=1,6
200  SAVG(I)=0.0

```

```

DO 210 I=1,5
  DO 210 L=1,MCN
    J=K+L
210  SAVG(I)=SAVG(I)+TSTS(I,J)
    DO 220 I=1,5
220  SAVG(I)=SAVG(I)/XMCN
    SMEAN=(SAVG(1)+SAVG(2)+SAVG(4))/3.
    IF(SAVG(5).EQ.0)GOTO 230
    RATIO=SMEAN/SAVG(5)
230  WRITE(6,240) (SAVG(I),I=1,5),EPS,SMEAN,RATIO
240  FORMAT(' AV',6(1PE12.4)'/ SIG MEAN='1PE10.3,' R='1PE10.3)
    DO 250 I=1,4
250  SAVG(I)=0.0
    DO 260 I=1,4
      DO 260 L=1,MCN
        J=K+L
260  SAVG(I)=SAVG(I)+EPSTN(I,J)
    DO 270 I=1,4
270  SAVG(I)=SAVG(I)/XMCN

```

```

.....
*
*   Writes plastic strains.
*
*
.....

```

```

      WRITE(6,280) (SAVG(I),I=1,4)
280  FORMAT(3X,4E12.4,' (PLASTIC STRAINS) ')
290  CONTINUE
    IF(MOUT.EQ.3)RETURN

```

```

.....
*
*   Writes residual forces at the nodes.
*
*
.....

```

```

      WRITE(6,300)
300  FORMAT(' RESIDUAL FORCES AT NODAL POINTS'/
  I   5X,'NODE NO',9X,'X VALUE',9X,'Y VALUE')
      DO 310 J=1,NP
        N=2*J
310  WRITE(6,320)J,R1(N-1),R1(N)
320  FORMAT(110,3E20.6)

```

```

RETURN
END

```

```

-----
-----
*
*      Subroutine PDISP:
*      Initializes the displacement vector according to number of
*      loading increments.
*
*      Written:
*      Last modified: MAR 19 1984
*
-----
-----

```

```

*
*      Called by: SCRUBS
*
-----
-----

```

SUBROUTINE PDISP(L)

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE
COMMON/GEF/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1  US(400,2)

IF(L-1)10,60,30
10  IC=NB*NDF
    DO 20 I=1,IC
20  U(I)=0.
    RETURN
30  FACT=FAC(L)
    IC=1
    NZ=10**(NDF-1)
    DO 50 I=1,NB
        JZ=NFIX(I)
        IZ=NZ
        DO 50 J=1,NDF
            IF(JZ.LT.IZ) GO TO 40
            U(IC)=US(I,J)*FACT

```

```

        IC=IC+1
        JZ=JZ-IZ
40      IZ=IZ/10
50      CONTINUE
        RETURN
60      FACT=FAC(INC)
        IC=0
        DO 70 I=1,NB
            DO 70 J=1,NDF
                IC=IC+1
70      U(IC)=US(I,J)*FACT

        RETURN
        END

```

```

-----
-----
*
*
*      Subroutine READF:
*      Reads the mining parameters: which elements, and when.
*
*      Written:
*      Last modified: JUL 24 1984
*
-----
-----

```

```

*
*      Called by: SCRUBS
*
-----
-----

```

```

SUBROUTINE READF(NRFF)

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

```

COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10)

```

```

-----
*
*
*      Reads number of elements to be mined per increment.
*
-----

```

```

10      READ (5,10)(NME(I),I=1,NRFF)
        FORMAT(25I5)

```

```

WRITE(6,20)
20  FORMAT(' LIST OF FAILURE(MINING) SEQUENCE '/
1   ' INCREMENT NUMBER OF ELEMENTS ',15X,'ELEMENT LIST')

```

```

-----
•
•   Reads the element numbers.
•
-----

```

```

DO 50 I=1,NRFF
  NL=NME(I)
  IF(NL.GT.25)GOTO 30
  READ(5,10) (MINES(I,K),K=1,NL)
  WRITE(6,25) (I,NL,(MINES(I,K),K=1,NL))
25  FORMAT(16I5)
  GOTO 50
30  READ(5,10) (MINES(I,K),K=1,25)
  READ(5,10) (MINES(I,K),K=26,NL)
  WRITE(6,25) (I,NL,(MINES(I,K),K=1,14))
  IDUM=(NL+2)/16
  IY=1
  DO 40 IZ=1,IDUM
    IY=IY+14
    IYY=IY+13
    IF(IYY.GT.NL)IYY=NL
40  WRITE(6,45) (MINES(I,K),K=IY,IYY)
45  FORMAT(10X,14I5)
50  CONTINUE

RETURN
END

```

```

-----
-----
•
• Subroutine REMLD:
• Removes gravity load from failed elements.
•

```

```

• Written:
• Last modified: JUL 19 1984
•

```

```

-----
-----
•
• Called by: FTEST
•

```

```

-----
-----
•
• Subroutines called: SFRAUX
•
-----
-----

```

SUBROUTINE REMLD(M)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

DIMENSION R(3)

DIMENSION NOPL(4),PS(4,2),PQ(2),DJ(2),DENS(10)

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF, LV,NPP,ICS,
1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO, PWORK,
3 TPWORK

```

```

COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)

```

```

COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN

```

```

COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST

```

```

1 ,TABSTN(10,15),TABSTR(10,15)

```

```

COMMON/LDS/R1(2000),RL(2000),RT(2000)

```

```

COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),

```

```

1 NOPE(12),DISE(2,12),RP,RPB,XYP(3)

```

```

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)

```

```

1 ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)

```

```

COMMON/GRAV/GRAV

```

GUSH=0.0

L=IMAT(M)

IF(L.GT.NMAT)L=L-100


```

THICK=ORT(L,6)
DENSE=DENS(L)
C    PQ(1)=PQ(2)=0.
    PQ(1)=0.
    PQ(2)=0.
    IF(DENSE.EQ 0.)GO TO 60
10   NASH=NCN*(M-1)
    DO 30 K=1,NCN
        MASH=NOP(NASH+K)
        DO 20 I=1,NCORD
20   XYE(I,K)=CORD(MASH,I)
30   NOPE(K)=MASH
    DO 50 IGAUS=1,NGAUS
        DO 50 JGAUS=1,NGAUS
            G=XG(IGAUS)
            H=XG(JGAUS)
            CALL SFR(G,H)
            CALL AUX(M)
            DV=DET*CG(IGAUS)*CG(JGAUS)
            IF(NPP.EQ 2)DV=DV*RP*6.28318530718
            IF(NCORD.EQ 3)DV=DV*XYP(3)
            IF(THICK.NE 0.)DV=DV*THICK
            PQ(1)=PQ(1)+DENSE*GUSH*XYP(1)*GRAV*FACTOR
        DO 40 I=1,NCN
            IC=(NOPE(I)-1)*2
            DO 40 K=1,2
                IC=IC+1
C    RL(IC)=RL(IC)-PQ(K)*P(I)*DV
        RT(IC)=RT(IC)-PQ(K)*P(I)*DV
40   CONTINUE
50   CONTINUE
60   CONTINUE

RETURN
END

```

•
• Subroutine RESIDUE:
• Determines element failure, rubbleization, and residual.

•
• Written:
• Last modified JUL 19 1984

•
• Called by: SCRUBS

•
• Subroutines called:MODSFR
• AUXLINEARJOINTM
• INVARNFLOWFTEST
• CONRUBDROP

SUBROUTINE RESIDUE (TIME1,IFCOUNT,MTGPY)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3 TPWORK
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1 US(400,2)
COMMON/LDS/R1(2000),RL(2000),RT(2000)
COMMON/BOTTOM/ AK
COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA
COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST
1 ,TABSTN(10,15),TABSTR(10,15)
COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),
1 NOPE(12),DISE(2,12),RP,RPB,XYP(3)
COMMON/INV/SMEAN,STJ2,STJ3,SIGMA,PHI,APHI,STN,CPhi,YIELD
COMMON/FLW/ABETA,AA(4),DD(4)

```

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1  ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
2  ,SIG(4),PSIG(4),DSIG(4),EP(4),NG(16),NGSTP(16)
   DIMENSION STRN(4),STRS(4),TSIG(4)
   DIMENSION SAVG(5)
   REAL TIME1,TIME2,TIME3

```

```

NECN=NE*NCN
READ (8)(NOP(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1  TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD),I=1,NP),(((TSTS(I,J),
2  EPSTN(I,J)),I=1,5),J=1,NSTORE)
   REWIND 8
   IFCOUNT=0
   IF(IT.GT.1) GO TO 20

```

•

• For initial iteration step set increment equal to zero.

•

```

DO 10 I=1,2
   DO 10 J=1,NP
10  DISN(I,J)=0.
20  CONTINUE

```

•

• Update coordinates, total displacements, and displacement increment.

•

```

DO 30 I = 1,NDF
   DO 30 J=1,NP
       GASH=DIS(I,J)
       CORD(J,I)=CORD(J,I)+GASH
       TDIS(I,J)=TDIS(I,J)+GASH
       DISN(I,J)=DISN(I,J)+GASH
30  CONTINUE
   DO 40 J=1,NSZF
40  R1(J)=0.
       PWORK=0.
       WRITE(6,50)FACTOR,IT
50  FORMAT(' LFACTOR  =',F7.3,2X,'ITERATION NO =',I3)
       MTGPY=0

```

Control loop on all elements.

```
DO 360 MNEW=1,NE
  MASH=MNEW-1
  NOSH=MASH*NCN
  MOSH=MASH*MCN
```

Find coordinates and connectivity.

```
DO 70 K=1,NCN
  MASH=NOP(NOSH+K)
  DO 60 I=1,NCORD
    XYE(I,K)=CORD(MASH,I)
  IF(1.GT.2) GO TO 60
  DISE(I,K)= DIS(I,MASH)
60   CONTINUE
    NOPE(K)=MASH
70   CONTINUE
  L=IMAT(MNEW)
  IF(L.GT.NMAT)L=L-100
```

Get elasticity matrix.

```
CALL MOD(L,1)
THICK=ORT(L,6)
IF(THICK.EQ.0.)THICK=1.
NPLST=0
IJG=0
```

Loop on integration points.

```

.....
DO 280 IGAUS=1,MGAUS
  DO 280 JGAUS=1,MGAUS
    IJG=IJG+1
    NG(IJG)=0
    NGSTP(IJG)=0
    G=XMG(IGAUS)
    H=XMG(JGAUS)

```

```

.....
*
*   Compute shape functions and derivatives.
*
.....

```

```

CALL SFR(G,H)
CALL AUX(MNEW)
DV=DET*CMG(IGAUS)*CMG(JGAUS)*THICK
IF(NPP.EQ.2)DV=DV*RP*6.283185307179586
IF(NCORD.EQ.3)DV=DV*XYP(3)
MOSH=MOSH+1

```

```

.....
*
*   Compute elastic strains and stresses in element.
*
.....

```

```

CALL LINEAR(STRN,STRS,W)
BRING=1.
IF(NL.NE.1) GO TO 90

```

```

.....
*
*   If element rubbleizes, set stresses to zero.
*
.....

```

```

IF(BULK(MNEW,1) EQ 0.0) GO TO 90
DO 80 J=1,4
80   STRS(J)=0.000001
90   CONTINUE
IF(BULK(MNEW,1).NE.0.0) GO TO 100

```

```

-----
*
*
*   For jointed media material find incremental stresses and residuals
*   in subroutine JOINTM.
*
-----

```

```

      IF(ORT(L,2) GE 0) GOTO 100
      CALL JOINTM(L,STRN,STRS,MOSH,YIELD,PSTRN)

```

```

-----
*
*
*   Set incremental stresses equal to elastic strains.
*
-----

```

```

100      DO 110 I=1,4
          DSIG(I)=STRS(I)
          EP(I)=EPSTN(I,MOSH)
C110     TSIG(I)=SIG(I)=TSTS(I,MOSH)
          TEMP=TSTS(I,MOSH)
          TSIG(I)=TEMP
          SIG(I)=TEMP
110      CONTINUE
          IF (NALGO.EQ.0) GO TO 240
          PSTRN=EPSTN(5,MOSH)
          PSTY=TSTS(5,MOSH)

```

```

-----
*
*
*   Add incremental stress to total stress.
*
-----

```

```

120      DO 120 I=1,4
          TSIG(I)=TSIG(I)+STRS(I)
          IF(ORT(L,2) LT 0) GOTO 240
          CALL INVAR (TSIG)
          RD=1.
          DF=YST-PSTY
          IF(DF LE 0.0) GO TO 130
          DF1=YIELD-YST
          IF(DF1 LE 0.0) GO TO 240
          RD=DF1/(DF+DF1)
          GO TO 140

```

```

130          DF1=YIELD-PSTY
            IF(DF1 LE.0.0) GO TO 240

```

```

-----
*
*      Compute plastic step.
*
-----

```

```

140          MSTEP=DF1*8.0/YST+1.
            ASTEP=MSTEP
            RD1=1.-RD
            DO 150 I=1,4
C            TSIG(I)=PSIG(I)+SIG(I)+RD1*STRS(I)
            TEMP=SIG(I)+RD1*STRS(I)
            TSIG(I)=TEMP
            PSIG(I)=TEMP
150          STRS(I)=RD*STRS(I)/ASTEP
            TLEMDA=0.
            NG(IJG)=MOSH
            NGSTP(IJG)=MSTEP
            NPLST=NPLST+1
            DO 200 ISTEP=1,MSTEP
                CALL INVAR (TSIG)

```

```

-----
*
*      Find position on effective curve.
*
-----

```

```

            MASH=0.
160          MASH=MASH+1
            IF(PSTRN LT TABSTN(L,MASH)) GO TO 170
            GO TO 160
170          NASH=MASH
            MASH=NASH-1
            ETAB=TABSTN(L,NASH)-TABSTN(L,MASH)
            HM=(TABSTR(L,NASH)-TABSTR(L,MASH))/ETAB

```

```

-----
*
*      Compute AA,ABETA,PD,TSIG.
*
-----

```

```

CALL NFLOW(TSIG)
  GASH=0.
  DO 180 I=1,4
180   GASH=GASH+AA(I)*STRS(I)
      DLEMDA=GASH*ABETA
      IF(DLEMDA.LT.0.) DLEMDA=0.
      GASH=0.
      DO 190 I=1,4
          GASH=GASH+AA(I)*PSIG(I)
          EP(I)=EP(I)+AA(I)*DLEMDA
C190   TSIG(I)=PSIG(I)=PSIG(I)+STRS(I)-DLEMDA*DD(I)
      TEMP=PSIG(I)+STRS(I)-DLEMDA*DD(I)
      PSIG(I)=TEMP
      TSIG(I)=TEMP
190   CONTINUE
      GASH=GASH*DLEMDA
      PSTRN=PSTRN+GASH/YIELD
      PWORK=PWORK+GASH*DV
      TLEMDA=TLEMDA+DLEMDA
200   CONTINUE
      DO 210 I=1,4
          GASH=TSIG(I)-SIG(I)
          PSIG(I)=DSIG(I)-GASH
210   DSIG(I)=GASH
      CALL INVAR (TSIG)
      MASH=0
220   MASH=MASH+1
      IF(PSTRN.LT.TABSTN(L,MASH)) GO TO 230
      GO TO 220
230   NASH=MASH
      MASH=MASH-1
      ETAB=TABSTN(L,NASH)-TABSTN(L,MASH)
      GASH=(PSTRN-TABSTN(L,MASH))/ETAB
      MASH=MASH-1
      CYIELD=TABSTR(L,MASH+1)*(1.-GASH)+GASH*
1   TABSTR(L,MASH+2)
      IF(YIELD.GT.CYIELD)BRING=-CYIELD/YIELD

```

•
•
• Add Jaumann stress to DSIG.
•
•

240 CONTINUE


```

DSIG(1)=DSIG(1)+2.*W*TSTS(1,MOSH)
DSIG(2)=DSIG(2)-2.*W*TSTS(2,MOSH)
DSIG(3)=DSIG(3)+W*(TSTS(2,MOSH)-TSTS(1,MOSH))

```

•

• Add increment to total stress.

•

```

DO 250 I=1,4
    GASH=BRING*(SIG(I)+DSIG(I))
    EPSTN(I,MOSH)=EP(I)
C250    STRS(I)=TSTS(I,MOSH)=GASH
        STRS(I)=GASH
        TSTS(I,MOSH)=GASH
250    CONTINUE
        YIELD=BRING*YIELD
        EPSTN(5,MOSH)=PSTRN
        TSTS(5,MOSH)=YIELD
DO 260 I=1,ICS
260    STRS(I)=DV*STRS(I)

```

•

• Compute residual.

•

```

DO 270 K=1,NCN
    GASH=0.
    IF(NPP.EQ.2)GASH=P(K)*STRS(4)/RP
    MASH=2*NOPE(K)-1
    NASH=MASH+1
    R1(MASH)=R1(MASH)+DI(1,K)*STRS(1)+
1      DI(2,K)*STRS(3)+GASH
    R1(NASH)=R1(NASH)+DI(1,K)*STRS(3)+
1      DI(2,K)*STRS(2)
270    CONTINUE
280    CONTINUE
    IF(NL.NE.1) GO TO 350

```

•

• Rubble failure logic.

```

•
•-----
DO 290 I=1,5
290 SAVG(I)=0.0
JJ=MCN*(MNEW-1)
DO 300 I=1,MCN
JJ1=JJ+I
DO 300 J=1,5
300 SAVG(J)=SAVG(J)+TSTS(J,JJ1)
DO 310 I=1,5
310 SAVG(I)=SAVG(I)*.25
MATNO=IMAT(MNEW)
IF(MATNO GT.100) MATNO=MATNO-100
CALL FTEST(MNEW,SAVG,MATNO,IFLFG)
IFCOUNT=IFCOUNT+IFLFG
IF(IFLFG.EQ.0) GO TO 350

```

```

•-----
•
• If element has failed, adjust R1 array and set coordinates,
• displacements and stresses.
•

```

```

• First check for continuum/rubble interaction.
•
•-----

```

```

CALL CONRUB(SIGRES,MNEW,NMAT)
INDEX=(MNEW-1)*MCN
DO340 IGAUS=1,MGAUS
DO340 JGAUS=1,MGAUS
G=XMG(IGAUS)
H=XMG(JGAUS)
CALL SFR(G,H)
CALL AUX(MNEW)
DV=DET*CMG(IGAUS)*CMG(JGAUS)*THICK
IF(NPP.EQ.2)DV=DV*RP*6.283185307179586
IF(NCORD.EQ.3)DV=DV*XYP(3)
INDEX=INDEX+1
DO 320 I=1,ICS
STRS(I)=DV*TSTS(I,INDEX)
320 TSTS(I,INDEX)=SIGRES
TSTS(3,INDEX)=0.0
TSTS(5,INDEX)=1.73205*SIGRES
DO 330 K=1,NCN

```

```

      GASH=0
      IF(NPP.EQ.2)GASH=P(K)*STRS(4)/RP
      MASH=2*NOPE(K)-1
      NASH=MASH+1
      R1(MASH)=R1(MASH)-DI(1,K)*STRS(1)-
1     DI(2,K)*STRS(3)-GASH
      R1(NASH)=R1(NASH)-DI(1,K)*STRS(3)-DI(2,K)*STRS(2)
330    CONTINUE
340    CONTINUE
350    CONTINUE
      MTGPY=MTGPY+NPLST
360    CONTINUE
      IF(NL.EQ.1) CALL DROP(NE,NP,AK)
      WRITE(8)(NOF(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1     TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD ),I=1,NP),(((TSTS(I,J),
2     EPSTN(I,J)),I=1,5),J=1,NSTORE)
      REWIND 8
      WRITE(6,370)MTGPY,IFCOUNT
370    FORMAT(' TOTAL GAUSS POINTS YIELDED DURING ITERATION =',I5/
1     ' TOTAL NUMBER OF ELEMENTS RUBBLEIZED DURING ITERATION =',I5)
      TIME4=SECNDS(TIME1)
      WRITE(6,380)TIME4
380    FORMAT(' RESULTANT FORCES OBTAINED AT',F10.3)

      RETURN
      END

```

```

-----
-----
*
*
*       Subroutine RESOLV:
*       Updates stiffness equation solution.
*
*
*       Written:
*       Last modified: JUL 18 1984
*
-----
-----
*
*       Called by: SCRUBS
*
-----
-----

```

SUBROUTINE RESOLV

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

3 TPWORK

COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),

1 US(400,2)

COMMON/LDS/R1(2000),RL(2000),RT(2000)

COMMON/SCR/SK(20000) ,R

READ(4) NTP ,(SK(I),I=NSK,NTP)

NBUF=NSK-1

ND=1

NNP=1

DO 60 L=1,NP

DO 50 I=1,NDF

IC=I+NDF*(L-1)

R=R1(IC)

NBUF=NBUF+1

NSZ=SK(NBUF)

NBUF=NBUF+1

IF(SK(NBUF) GT 0.) GO TO 10

RS=-R

R=-U(ND)

ND=ND+1

R1(IC)=SK(NBUF)*R+RS

GO TO 20

10 CONTINUE

R1(IC)=SK(NBUF)*R

20 CONTINUE

IF(L+I-NP-NDF) 30,70,30

30 NBUF=NBUF+1

DO 40 J=1,NSZ

IK=IC+J

R1(IK)=R1(IK)-SK(NBUF)*R

40 NBUF=NBUF+1

NBUF=NBUF+1

IF(NBUF.LT.NTP) GO TO 50

READ(4) NTP ,(SK(II),II=NSK,NTP)

NBUF=NSK-1

NNP=0

50 NNP=NNP+1

60 CONTINUE

70 RETURN

END

```

-----
-----
*
*      Subroutine ROSB:
*      Adjusts stiffness matrix.
*
*      Written:
*      Last modified: JUL 14 1984
*
-----
-----
*
*      Called by: STIFM
*
-----
-----

```

SUBROUTINE ROSB

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD

COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),

1 US(400,2)

COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),

1 NOPE(12),DISE(2,12),RP,RPB,XYP(3)

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)

1 ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)

2 ,COJ(2,2),COJIN(2,2),B(4,24),ESTIFM(24,24),DVD(4,4),DVDB(4)

DO 60 I=1,NCN

DO 10 M=1,NB

IF(NOPE(I) EQ NBC(M)) GO TO 20

10 CONTINUE

GO TO 60

20 IF(ANG(M) EQ 0.) GOTO 60

GASH=ANG(M)*.017453292

CS=COS(GASH)

TN=TAN(GASH)

MASH=2*1-I

NASH=MASH+1

DO 30 K=1,LV

STFUC=ESTIFM(MASH,K)*CS

```

      STFVC=ESTIFM(NASH,K)*CS
      ESTIFM(MASH,K)=STFUC+STFVC*TN
      ESTIFM(NASH,K)=STFVC-STFUC*TN
30    CONTINUE
      DO 40 K=MASH,NASH
          STFUC=ESTIFM(K,MASH)*CS
          STFVC=ESTIFM(K,NASH)*CS
          ESTIFM(K,MASH)=STFUC+STFVC*TN
          ESTIFM(K,NASH)=STFVC-STFUC*TN
40    CONTINUE
      DO 50 J=MASH,NASH
          DO 50 K=1,LV
              ESTIFM(K,J)=ESTIFM(J,K)
50    CONTINUE
60    CONTINUE

      RETURN
      END

```

```

-----
-----

```

```

*
*
*       Subroutine ROTATE:
*       This subroutine rotates 2D stress or strain vector by angle THETA.
*
*       Written:
*       Last modified: MAR 19 1984
*

```

```

-----
-----

```

```

*
*       Called by: JOINTM
*

```

```

-----
-----

```

SUBROUTINE ROTATE(A,B,THETA)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

DIMENSION A(4),B(4)

C=DCOS(THETA)

S=DSIN(THETA)

B(1)=A(1)*C+C*A(2)*S-S*2.*A(3)*C*S

B(2)=A(1)*S+S*A(2)*C+C*2.*A(3)*C*S

$$B(3)=A(1)\cdot C\cdot S+A(2)\cdot C\cdot S+A(3)\cdot C\cdot C+A(3)\cdot S\cdot S$$

$$B(4)=A(4)$$

RETURN

END

```

-----
-----
*
*      Subroutine RUBDRAW:
*      Writes the rubble file RUB.MOV.
*
*      Written:
*      Last modified: JUL 19 1984
*
-----
-----
*
*      Called by: SCRUBS
*
-----
-----

```

SUBROUTINE RUBDRAW(NUMEL,TT,NUMNP)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3  TPWORK,KNE
DIMENSION EPX4(20,1),R(1),Z(1),IX(5,1)
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1  ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)
DIMENSION IXAR(4000)
DIMENSION IXN(20),RN(12),ZN(12),ZETA(12),ETA(12)
REAL TIME1,TIME2,TIME3
DIMENSION IDUM(12)
REAL CORDDUM(1000,2),R1,R2,R3,R4,Z1,Z2,Z3,Z4,ZERO

DATA IXN/1,2,6,-4,2,3,8,-6,4,5,10,-9,5,7,11,-10,7,8,12,-11/
DATA ZETA/-1, 0, 1,-1,-5, 0, -5, 1,-1,-5, 5, 1/
DATA ETA/-1,-1,-1, 0, 0, 0, 0, 0, 1, 1, 1, 1/
DATA IDUM/8,16,24,31,39,47,55,62,70,78,86,94/

```

```

NP1=1
NP2=NUMEL
IBEGIN=NUMEL
ICNT=1

```

```

•
•   Eliminates plotting of boundary around rubble.
•

```

```

IR1=0
DO 20 I=1,NUMEL
  IF(BULK(I,1).EQ.0.0) GO TO 10
  NP2=NP2-1
  ICNT=ICNT+1
  GO TO 20
10  CONTINUE
  IR2=4*NSFR*(I-1)
  IXAR(IR1+1)=NOP(IR2+1)
  IXAR(IR1+2)=NOP(IR2+1+NSFR)
  IXAR(IR1+3)=NOP(IR2+1+NSFR*2)
  IXAR(IR1+4)=-NOP(IR2+1+NSFR*3)
  IR1=IR1+4*NSFR
20  CONTINUE
  WRITE(14,30) TT,ICNT
30  FORMAT (E15.5,15)
  NONE=1
  NCON=4*NP2*NSFR
  WRITE(14,60) NONE,NUMNP,NP2,NCON
  WRITE (14,60) NP1,NP2
  ZERO=0.0
  DO 40 I=1,NUMNP
    CORDDUM(I,1)=SNGL(CORD(I,1))
40  CORDDUM(I,2)=SNGL(CORD(I,2))
    WRITE(14,50) ((CORDDUM(I,1),CORDDUM(I,2),ZERO),I=1,NUMNP)
50  FORMAT (6E12.5)
    WRITE(14,60) (IXAR(I),I=1,NCON)
60  FORMAT (16I5)
    ICNT=0
    NP1=NP2
  DO 100 I=1,NUMEL
    IF (BULK(I,1) EQ 0.0) GO TO 100
    ICNT=ICNT+1
    NP1=NP2+1

```



```

NP2=NP1+4*NSFR
INDEX=4*(I-1)*NSFR
I1=NOP(INDEX+1)
I2=NOP(INDEX+1+NSFR)
I3=NOP(INDEX+1+NSFR*2)
I4=NOP(INDEX+1+NSFR*3)
Z1=SNGL(CORD(I1,2))
Z2=SNGL(CORD(I2,2))
Z3=SNGL(CORD(I3,2))
Z4=SNGL(CORD(I4,2))
R1=SNGL(CORD(I1,1))
R2=SNGL(CORD(I2,1))
R3=SNGL(CORD(I3,1))
R4=SNGL(CORD(I4,1))
ISWSW=0
IF(ISWSW.EQ.0) GO TO 80
IF(BULK(I,1),EQ.2.0) GO TO 80
DO 70 KK=1,12
    IF(I.EQ.IDUM(KK)) ISWSW=1
70    CONTINUE

```

•
•
•
•
•

Leaves gap for unconsolidated rubble.

```

ZMAX=AMAX1(Z1,Z2,Z3,Z4)
ZMIN=AMIN1(Z1,Z2,Z3,Z4)
IELE=IMAT(I)
ALPH=ORT(IELE,8)
RMAX=AMAX1(R1,R2,R3,R4)
RMIN=AMIN1(R1,R2,R3,R4)
C    RDEL=RMAX-RMIN
C    AREA=BULK(I,2)
C    ANEW=AREA/ALPH
C    ZDEL=ANEW/RDEL
C    ZNEW=ZMIN+ZDEL
C    ZAVG=(Z1+Z2+Z3+Z4)*.25
C1=CORD(I1,2)-TDIS(2,I1)
C2=CORD(I2,2)-TDIS(2,I2)
C3=CORD(I3,2)-TDIS(2,I3)
C4=CORD(I4,2)-TDIS(2,I4)
ZD1=ABS(C3-C1)
ZD2=ABS(C4-C2)

```

```

      ZDST=5*(ZD1+ZD2)
      ZLEN=ZDST/ALPH
      ZNEW=ZMIN+ZLEN
      ZNEW=SNGL(ZNEW)
      IF(Z1.GT.ZAVG.AND.FLAG(11).GT.1.0) Z1=ZNEW
      IF(Z2.GT.ZAVG.AND.FLAG(12).GT.1.0) Z2=ZNEW
      IF(Z3.GT.ZAVG.AND.FLAG(13).GT.1.0) Z3=ZNEW
      IF(Z4.GT.ZAVG.AND.FLAG(14).GT.1.0) Z4=ZNEW
80     CONTINUE

```

```

-----
*
*     Computes nodal coordinates for intra-element rubble.
*
-----

```

```

      DO 90 INDEX=1,12
          F1=1.-ZETA(INDEX)
          F2=1.+ZETA(INDEX)
          G1=1.-ETA(INDEX)
          G2=1.+ETA(INDEX)
          RN(INDEX)=.25*(F1*G1*R1+F2*G1*R2+F2*G2*R3+F1*G2*R4)
          ZN(INDEX)=.25*(F1*G1*Z1+F2*G1*Z2+F2*G2*Z3+F1*G2*Z4)
90     CONTINUE
      NJ=12
      NPT=5
      NCON=20
      WRITE(14,60) NONE,NJ,NPT,NCON
      WRITE(14,60) NP1,NP2
      WRITE (14,50) ((RN(J),ZN(J),ZERO),J=1,12)
      WRITE (14,60) (IXN(J),J=1,20)
100    CONTINUE

      RETURN
      END

```

```

-----
-----
*
*
*      Subroutine RZBASE:
*      This subroutine finds the base R and Z dimensions for fall
*      calculations.
*
*      Written:
*      Last modified: MAR 19 1984
*
-----
-----

```

```

-----
-----
*
*      Called by: DROP
*
-----
-----

```

```

-----
-----
*
*      Subroutines called: EXIT
*
-----
-----

```

```

SUBROUTINE RZBASE(I,IBOT,I1,I2,I3,I4,R,Z,RLEN,ZBASE)

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

```

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1 ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)

```

```

R1=CORD(I1,1)

```

```

R2=CORD(I2,1)

```

```

R3=CORD(I3,1)

```

```

R4=CORD(I4,1)

```

```

Z1=CORD(I1,2)

```

```

Z2=CORD(I2,2)

```

```

Z3=CORD(I3,2)

```

```

Z4=CORD(I4,2)

```

```

RAVG=(R1+R2+R3+R4)*.25

```

```

ZAVG=(Z1+Z2+Z3+Z4)*.25

```

```

RMX=0.0

```

```

IF(R1.GT.RAVG)RMX=RMX+R1

```

```

IF(R2.GT.RAVG)RMX=RMX+R2

```

```

IF(R3.GT.RAVG)RMX=RMX+R3

```

```

IF(R4.GT.RAVG)RMX=RMX+R4

```

```

RMX=RMX*.5

```

```

RMN=0.0
IF(R1.LT.RAVG)RMN=RMN+R1
IF(R2.LT.RAVG)RMN=RMN+R2
IF(R3.LT.RAVG)RMN=RMN+R3
IF(R4.LT.RAVG)RMN=RMN+R4
RMN=RMN*.5
RLEN=RMX-RMN

```

```

-----
*
*
*   Determines ZBASE node.
*
*
-----

```

```

IF(I.NE.I1) GO TO 10
ITEST1=I2
ITEST2=I4
GO TO 60
10  IF(I.NE.I2) GO TO 20
    ITEST1=I1
    ITEST2=I3
    GO TO 60
20  IF(I.NE.I3) GO TO 30
    ITEST1=I2
    ITEST2=I4
    GO TO 60
30  IF(I.NE.I4) GO TO 40
    ITEST1=I3
    ITEST2=I1
    GO TO 60
40  WRITE(6,50) I,I,BOT
50  FORMAT('  NODE NUMBER ',I5,' IS NOT OF ELEMENT ',I5/)
    CALL EXIT
60  CONTINUE
    IF(CORD(ITEST1,2).LT.CORD(ITEST2,2)) GO TO 70

```

```

-----
*
*
*   ITEST2 is ZBASE node.
*
*
-----

```

```

ZBASE=CORD(ITEST2,2)
RETURN

```

•-----
•
• ITEST1 is ZBASE node.
•
•-----

70 ZBASE=CORD(ITEST1,2)

 RETURN

 END

•
• Main SCRUBS: Program designed to compute the failure, collapse,
• and resulting subsidence of geologic materials.
•

• Elastic Plastic analysis of plane stress/strain and axisymmetric
• problems for linear, parabolic, and cubic elements.
•

• Written by:

• G. NAYAK.....University College of Swansea
•

• Compiled for CDC 7600 by:

• C. ANDERSON.....Los Alamos Scientific Laboratory

• RV BROWNING.....Los Alamos Scientific Laboratory
•

• Modified to treat finite strain, large deformation, and
• geotechnical rubble formation by:

• S.E. BENZLEY.....Brigham Young University
•

•
• Called by SCRUBSV (a file driver)
•

• Subroutines called: SOLVEREADF

• ZONEOUTPUTLDATA

• GDATAETPDISP

• FLAGETFORMRESIDUE

• RESOLVBSUBSTIFM

• CONVGRUBDRAWDATAALST (not used at present)
•

SUBROUTINE SCRUBS

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/ELPRNT/ NLP,NPEL(50),NONODE

COMMON/DUMM/ SAVG(4),SPLT(4,1000)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

```

2      NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3      TPWORK,KNE
      COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
      COMMON/BOUN/NBC(400),NFI(400),U(800),ANG(400),TRC(800),
1      US(400,2)
      COMMON/LDS/R1(2000),RL(2000),RT(2000)
      COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
      COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST
1      ,TABSTN(10,15),TABSTR(10,15)
      COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),
1      CORD(1000,2)
1      ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),
2      DUMMY(1000)
      COMMON/DLIST/ LN,LNCT,TITL (20)
      COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),
1      GAMMA
      COMMON/BOTTOM/ AK
      COMMON/QMESH/NEL
      DIMENSION IRAY(2)
      REAL TINC,UDUM(2,1000),SPLTDUM(4,1000),CORDDUM(1000,2)
      REAL TIME1,TIME2,TIME3,TITLD(20)
      DATA LNCT/55/
      DATA IRAY(1),IRAY(2)/10,12/

C      CALL FTNBIN(1,2,IRAY)
C      REWIND 12
C      CALL DATALST(12,5,6)
      NRS=0

```

•
•
•
•
•

Opens and reads data from QMESH9.DAT file in subroutine ZONE.

```

      CALL ZONE(NRS)
      IF(NRS.NE.1) GO TO 20

```

•
•
•
•

Reads restart file, SCRUBS.RST.

```

      REWIND(2)

```

```

      READ(2) (TITLE(I),I=1,57),(FAC(I),I=1,1210),(NBC(I),I=1,1800),
1  (R1(I),I=1,6000),(XG(I),I=1,27),(D(I,J),I=1,4),J=1,4),
2  ((DIS(I,J),I=1,2),J=1,1000),LN,LNCT,(TITL (I),I=1,20),
3  (FLAG(I),I=1,3510),AK,NELP,(NPEL(I),I=1,51)
      NECN=NE*NCN
      READ(2) (NOP(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1  TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD ),I=1,NP),(((TSTS(I,J),
2  EPSTN(I,J)),I=1,5),J=1,NSTORE)
3  ,(TABSTN(I,J),I=1,10),J=1,15),(TABSTR(I,J),I=1,10),J=1,15)
      REWIND (8)
      WRITE(8)(NOP(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1  TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD ),I=1,NP),(((TSTS(I,J),
2  EPSTN(I,J)),I=1,5),J=1,NSTORE)
      REWIND (8)

```

Writes the solution controls to SCRUBS.LIS.

```

      WRITE(6,10) (TITLE(I),I=1,20)
10  FORMAT(' ',20A4,/' ***** THIS PROBLEM HAS BEEN RESTARTED '
1  '*****'/)
      WRITE(6,15) NP,NE,NB,NLD,NDF,NMAT,NSFR,NGAUS,NALGO,NPP,
1  NYIELD,NT,NL,NSIZ,NCORD,MGAUS
15  FORMAT('          CONTROL PARAMETERS'/
1  '    NUMBER OF NODAL POINTS-----',I3//
2  '    NUMBER OF ELEMENTS-----',I3//
3  '    NUMBER OF BOUNDARY POINTS----',I3//
4  '    NUMBER OF LOAD CASES-----',I2//
5  '    NUMBER OF DEGREES OF FREEDOM--',I2//
6  '    NUMBER OF MATERIALS-----',I2//
7  '    ORDER OF THE ELEMENTS-----',I2//
8  '    NUMBER OF GAUSS POINTS-----',I2//
9  '    SOLUTION ALGORITHM-----',I2//
A  '    GEOMETRY NUMBER-----',I2//
B  '    YIELD CONDITION NUMBER-----',I2//
C  '    NUMBER OF ARBITRARY STIFF ----',I2//
D  '    BLANK-----',I2//
E  '    BAND WIDTH (NOT NEEDED)-----',I2//
F  '    NUMBER OF COORDINATES-----',I2//
G  '    GAUSS PTS FOR. RES. CALC.----',I2)

      CALL OUTPUT
      GO TO 30

```


20 CONTINUE

 *
 * Reads NPROB, the number of problems, from SCRUBS.DAT, and
 * writes NPROB and the starting time to SCRUBS.LIS.
 *

```

25      READ(5,25) NPROB
        FORMAT(16I5)
        DO 760 NPR=1,NPROB
          TIME1=SECNDS(0.0)
          WRITE(6,26) NPR,TIME1
26      FORMAT(' PROBLEM NO.',I3,3X,'EXECUTION STARTED AT ',
1         F10.3)

```

 *
 * Reads the title from SCRUBS.DAT and writes it to SCRUBS.LIS.
 *

```

27      READ(5,27) (TITLE(I),I=1,20)
        FORMAT(20A4)
        WRITE(6,27) (TITLE(I),I=1,20)

```

 *
 * Reads solution control data from SCRUBS.DAT.
 *

```

        FACTOR=0.
        READ (5,25)NP,NE,NB,NLD,NDF,NMAT,NSFR,NGAUS,NALGO,NPP,
1        NYIELD,NT,NL,NSIZ,NCORD,MGAUS

```

 *
 * Opens RUB.MOV file if rubble flag is set equal to 1.
 *

```

1      IF(NL.EQ.1)OPEN(UNIT=14,TYPE='NEW',ACCESS='SEQUENTIAL',
        NAME='RUB.MOV')

```

Reads comment header from first line of QMESH9.DAT,
 then the number of elements (NE) and the
 number of nodal points (NP).

```

C      REWIND 9
      READ(9) KOMT
      IF(NEL.EQ.0) READ(9) KOMT
      READ(9) NE,NP
  
```

Writes the solution controls to SCRUBS.LIS.

```

      IF(NCORD.EQ.0) NCORD=2
      IF(MGAUS.EQ.0) MGAUS=NGAUS
      WRITE(6,15) NP,NE,NB,NLD,NDF,NMAT,NSFR,NGAUS,NALGO,NPP,
1      NYIELD,NT,NL,NSIZ,NCORD,MGAUS
  
```

Reads material properties.

```

30      CONTINUE
      DO 40 L=1,NMAT
          READ (5,35) N,(ORT(N,I),I=1,6)
35      FORMAT(110,6G10.4)
40      IF(ORT(N,2) LT 0.0) READ (5,50) (ORT(N,I),I=9,10)
50      FORMAT(2G12.5)
          WRITE(6,60)
60      FORMAT(' MATERIAL PROPERTIES'/
1      '  MATL  YOUNGS MOD   POIS RAT   YIELD'
2      ' HARD MOD ANGLE THICK ')
          DO 90 N=1,NMAT
              IF(NYIELD.NE.3) GO TO 70
  
```

•
 • Adjusts the yield stress and the yield angle for Von Mises
 • failure criteria.
 •

```

      C=ORT(N,3)
      THETA=ORT(N,5)
      SN=SIN(THETA)
      DNOM=1.73205080757*(3.-SN)
      ORT(N,3)=6.*C*COS(THETA)/DNOM
      ORT(N,5)=2.*SN/DNOM
70      WRITE(6,80)N,(ORT(N,I),I=1,6)
80      FORMAT(I10,G12.4,G12.4,2G12.4,3G9.4)
90      IF(ORT(N,2).LT.0.0)WRITE(6,100) (ORT(N,I),I=9,10)
100     FORMAT(' JOINTED MEDIA MATERIAL-- DELTA=',F8.4,
1      'GS=',E10.4)

```

•
 • Tabulated stress/plastic-strain data.
 •

```

      WRITE(6,110)
110     FORMAT(' TABULATED PLASTIC STRAIN-STRESS DATA')
      DO 180 N=1,NMAT
      TABSTR(N,1)=ORT(N,3)
      IF(ORT(N,4).LT.0.) GO TO 120
      NTAB=2
      TABSTN(N,2)=1.
      TABSTR(N,2)=TABSTR(N,1)+ORT(N,4)
      GO TO 140

```

•
 • Reads tabulated stress/strain data from SCRUBS.DAT.
 •

```

120     READ(5,35) NTAB
      READ(5,130)(TABSTR(N,I),I=2,NTAB)
130     FORMAT(10F8.0)
      READ(5,130) (TABSTN(N,I),I=2,NTAB)
140     CONTINUE

```

TABSTN(N,1)=0.0

Writes tabulated stress/strain data to SCRUBS.LIS.

```

WRITE(6,150)
150   FORMAT(' TABULATED STRESS VALUES')
      WRITE(6,160) (TABSTR(N,I),I=1,NTAB)
160   FORMAT(' ',15E9.3)
      WRITE(6,170)
170   FORMAT(' TABULATED STRAIN VALUES')
180   WRITE(6,190) (TABSTN(N,I),I=1,NTAB)
190   FORMAT(1X,15E9.3)
      IF(NRS.EQ.1) GO TO 290

```

Reads and sets bulking parameters.

```

IF(NL.EQ.0) GO TO 260
DO 200 L=1,NMAT
200   READ(5,35) N,(ORT(N,I),I=7,8)
      WRITE(6,210)
210   FORMAT(' BULKING PROPERTIES/' MATL FAILURE '
1     'STRESS ALPHA'/)
      DO 220 N=1,NMAT
220   WRITE(6,80),N,(ORT(N,I),I=7,8)
      READ(5,230) AK
230   FORMAT(E10.3)
      WRITE(6,240) AK
240   FORMAT(' ++++++++ LOWER BOUNDARY INDICATOR, AK =',E15.4)

```

Reads failure parameters, NRFF = number of load steps with defined
element removal.

```

READ(5,25) NRFF

```

```

WRITE(6,250) NRFF
250   FORMAT(' FAILURE READ FLAG (I.E. NUMBER OF LOAD '
1     'STEPSj = ',I5)
      IF(NRFF.NE.0) CALL READF(NRFF)
260   CONTINUE

```

```

•
•   Preset discs.
•
•
•

```

```

REWIND 3
REWIND 4
REWIND 8
ICS=3
IF(NPP.EQ.2)ICS=4
NCN=4*NSFR
MCN=MGAUS*MGAUS
NSZF=NP*NDF
LV=NCN*NDF
NSTORE=MCN*NE
WRITE(6,270)NCN,MCN,LV,NSZF,ICS,NSTORE
270   FORMAT(' NODES/ELEMENT=',I2,2X,' GAUSSIAN POINTS=',I3,
1     2X,'UNKNOWNNS/ELEMENT=',I3,2X,'TOTAL UNKNOWNNS=',I4,2X,
2     'DMATRIX SIZE=',I2,2X,'STRESS STORE=',I4)

```

```

•
•   Recomputes NSTORE if slip plane is added.
•
•
•

```

```

IIF=1
IF(NNP.GT.0)IIF=-1

```

```

•
•   Reads geometrical data.
•
•
•

```

```

CALL GDATA (IIF)

```

•
• Sets initial areas.
•

```

IF(NL EQ 0) GO TO 290
DO 280 N=1,NE
  K1=(N-1)*NSFR*4+1
  K2=K1+NSFR
  K3=K2+NSFR
  K4=K3+NSFR
  J1=NOP(K1)
  J2=NOP(K2)
  J3=NOP(K3)
  J4=NOP(K4)
  COE1=CORD(J2,1)-CORD(J4,1)
  COE2=CORD(J3,2)-CORD(J1,2)
  COE3=CORD(J3,1)-CORD(J1,1)
  COE4=CORD(J4,2)-CORD(J2,2)
  AREA= 5*(COE1*COE2+COE3*COE4)
  BULK(N,1)=0.
  BULK(N,2)=AREA
280  CONTINUE
290  CONTINUE

```

•
• Writes first record for plot tape output (SCRUBS.MOV).
•

```

DO 300 I=1,20
300  TITLD(I)=TITLE(I)
  WRITE(10) TITLD,NE,NP
  DO 310 I=1,NP
    CORDDUM(I,1)=SINGL(CORD(I,1))
310  CORDDUM(I,2)=SINGL(CORD(I,2))
  WRITE(10)(CORDDUM(I,1),I=1,NP),(CORDDUM(I,2),I=1,NP),
  I((NOP(NCN*(K-1)+M),K=1,NE),M=1,NCN),(IMAT(I),I=1,NE)
  IF(NRS EQ 1) GO TO 360

```

•
• Sets element spacing and checks bandwidth and stiffness dimensioning.
•

CALL SET

Inputs loads.

```

DO 750 ILD=1,NLD
  TTWORK=0.
  READ(5,25)ISTS,LDTYPE
  WRITE(6,320)ISTS,LDTYPE
320  FORMAT(' INITIAL STRESS COUNTER=',I2,2X,'LOAD TYPE '
1    COUNTER=',I2)
  NASH=0
  DO 330 J=1,NP
    DO 330 I=1,NDF
      NASH=NASH+1
C    330 TDIS(I,J)=RL(NASH)=DISN(I,J)=RT(NASH)=R1(NASH)=0>
      TDIS(I,J)=0.
      RL(NASH)=0.0
      DISN(I,J)=0.0
      RT(NASH)=0.0
      R1(NASH)=0.0
330  CONTINUE
  DO 340 J=1,NSTORE
    DO 340 I=1,5
      EPSTN(I,J)=0.
340  TSTS(I,J)=0.
      NECN=NE*NCN
      WRITE(8)(NOP(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1    TD:S(K,I),K=1,2),(CORD(I,J),J=1,NCORD ),I=1,NP),
2    (((TSTS(I,J),EPSTN(I,J)),I=1,5),J=1,NSTORE)
      REWIND 8
      NASH=0
      DO 350 I=1,NB
        DO 350 K=1,2
          NASH=NASH+1
350  TRC(NASH)=0.
      IF(LDTYPE EQ 2) GO TO 410
      CALL LDATA(IIF)
360  CONTINUE
      READ (5,25)NINC

```

```

WRITE(6,370) NINC
370  FORMAT(' NO. OF INCREMENTS =',I2/
1    ' INCREMENT  OUTPUT LD FACTOR')
READ (5,380)(FAC(I),I=1,NINC)
380  FORMAT(16F5.3)
READ (5,25)(NOUT(I),I=1,NINC)
DO 390 I=1,NINC
390  WRITE(6,400)I,NOUT(I),FAC(I)
400  FORMAT(2I10,F10.3)
GO TO 420
410  FACTOR=1.
420  CONTINUE
READ (5,35)NIT,CONFAC
WRITE(6,430)NIT,CONFAC
430  FORMAT(' NUMBER ITERATIONS=',I2,5X,'CONVERGENCE '
1    'FACTOR =',F10.2)
IF(NL.EQ.1.AND.NRS.NE.1) CALL FLAGE(NP,NE,5,IIF)

```

•
•
• Sets print controls.
•
•

```

READ(5,440) NELP,NONODE
440  FORMAT(16I5)
IF(NELP.LE.0) GO TO 470
READ(5,440) (NPEL(K),K=1,NELP)
WRITE(6,450)
450  FORMAT(' ELEMENTS TO BE PRINTED')
WRITE(6,460) (NPEL(K),K=1,NELP)
460  FORMAT(1X,26I5/26I5)
470  CONTINUE

```

•
• Begins loop for each increment.
•
•

```

DO 740 INC=1,NINC
TPWORK=0.
WRITE(6,480)INC
480  FORMAT(' LOAD INCREMENT NO. =',I3)
IF(LDTYPE.EQ.2) GO TO 500

```



```

GASH=FAC(INC)
FACTOR=FACTOR+GASH
NASH=0
DO 490 J=1,NP
    DO 490 I=1,NDF
        NASH=NASH+1
        GOSH=RL(NASH)*GASH
        RT(NASH)=RT(NASH)+GOSH
        R1(NASH)=R1(NASH)+GOSH
490    CONTINUE
    CALL TFORM
    GO TO 510
500    CALL LDATA(IIF)
510    IF(INC.GT.1 .AND. NALGO.NE.1) GO TO 530
    CALL PDISP(1)
    CALL STIFM
    TIME3=SECNDS(TIME1)
    WRITE(6,520)TIME3
520    FORMAT(' STIFFNESS FORMULATION FINISHED AT',F10.3)
    CALL SOLVE
    GO TO 540
530    CALL PDISP(INC)
    CALL RESOLV
540    CALL BSUB
    IF(NALGO.EQ.0) CALL RESIDUE (TIME1,IFCOUNT,MTGPY)
    IF(NALGO.EQ.0) GO TO 600
    CALL PDISP(0)

```

•
•
•
•
•

Begins loop for each iteration.

```

DO 580 IT=1,NIT
    CALL RESIDUE (TIME1,IFCOUNT,MTGPY)
    CALL CONVG(NCHECK)
    CALL TFORM
    IF(IT.EQ.1 .AND. NOUT(INC).GT.100)CALL OUTPUT
    IF(NCHECK.EQ.0) GO TO 600
    IF(NALGO.LT.2)GO TO 560
    IF(NALGO.GT.2)GO TO 550
    IF(IT.NE.1)GO TO 560
550    CONTINUE

```

```

.....
*
*       If no elements have yielded or rubblized, do not reform stiffness
*       matrix.
*
.....

```

```

      IICK=IFCOUNT+MTGPY
      IF(IICK.EQ.0) GO TO 560
          CALL STIFM
          TIME3=SECNDS(TIME1)
          WRITE(6,520)TIME3
          CALL SOLVE
          GO TO 570
560      CALL RESOLV
570      CALL BSUB
580      CONTINUE
          IT=NIT+1
          CALL RESIDUE (TIME1,IFCOUNT,MTGPY)
          CALL CONVG(NCHECK)
          WRITE(6,590) INC
590      FORMAT('      NO CONVERGENCE ON INCREMENT NO',I3)
          GO TO 760
600      CONTINUE
          CALL OUTPUT

```

```

.....
*
*       Write restart tape (SCRUBS RST).
*
.....

```

```

      REWIND(2)
      WRITE(2) (TITLE(I),I=1,57),(FAC(I),I=1,1210),(NBC(I),
1      I=1,1800),(R1(I),I=1,6000),(XG(I),I=1,27),((D(I,J),
2      I=1,4),J=1,4),((DIS(I,J),I=1,2),J=1,1000),LN,LNCT,
3      (TITL (I),I=1,20),(FLAG(I),I=1,3510),AK,NELP,
4      (NPEL(I),I=1,51)
          NECN=NE*NCN
          WRITE(2)(NOP(J),J=1,NECN),(IMAT(I),I=1,NE),
1      (((DISN(K,I),TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD ),
2      I=1,NP),(((TSTS(I,J),EPSTN(I,J)),I=1,5),J=1,NSTORE),
3      ((TABSTN(I,J),I=1,10),J=1,15),((TABSTR(I,J),I=1,10),
4      J=1,15)
          WRITE(6,610)

```

```

610          FORMAT(' * * * * RESTART FILE HAS BEEN '
1          'WRITTEN * * * *')
          IF(NALGO.EQ.0) GO TO 740
          TTWORK=TTWORK+TPWORK
          WRITE(6,620)INC,TTWORK
620          FORMAT(' INC NO ',I2.5X,'TOTAL PLASTIC WORK =' ,E12.4)
          TIME2=SECNDS(TIME1)
          WRITE(6,630) TIME2
630          FORMAT(4X,'EXECUTION FINISHED AT ',F10.3)

```

```

-----
*
*      Computes average stress in each element.
*
-----

```

```

          XMCN=MCN
          DO 670 M=1,NE
              K=MCN*(M-1)
              DO 640 I=1,4
640                 SAVG(I)=0.0
              DO 650 L=1,MCN
                  J=K+L
650                 SAVG(I)=SAVG(I)+TSTS(I,J)
              DO 660 I=1,4
660                 SAVG(I)=SAVG(I)/XMCN
                  SPLT(1,M)=SAVG(1)
                  SPLT(2,M)=SAVG(2)
                  SPLT(3,M)=SAVG(4)
                  SPLT(4,M)=SAVG(3)
670          CONTINUE

```

```

-----
*
*      Wntes displacements, stresses, and strains for plot tape
*      SCRUBS.MOV.
*
-----

```

```

          TINC=INC
          WRITE(10) TINC
          DO 680 I=1,NP
              UDUM(1,I)=SNGL(TDIS(1,I))
680          UDUM(2,I)=SNGL(TDIS(2,I))

```

```

        DO 690 J=1,NE
            DO 690 I=1,4
690      SPLTDUM(I,J)=SNGL(SPLT(I,J))
            WRITE(10) (UDUM(1,I),UDUM(2,I),I=1,NP),
1          (UDUM(1,I),UDUM(2,I),I=1,NP),
2          (UDUM(1,I),UDUM(2,I),I=1,NP)
            WRITE(10) ((SPLTDUM(I,J),J=1,NE),I=1,4)

```

```

.....
*
*
*   Writes rubble file (RUB.MOV).
*
*
.....

```

```

        IF(NL.EQ.1) CALL RUBDRAW(NE,TINC,NP)

```

```

.....
*
*
*   Computes average strain in each element.
*
*
.....

```

```

        DO 730 M=1,NE
            K=MCN*(M-1)
            DO 700 I=1,4
700      SAVG(I)=0.0
            DO 710 I=1,4
                DO 710 L=1,MCN
                    J=K+L
710      SAVG(I)=SAVG(I)+EPSTN(I,J)
                DO 720 I=1,4
720      SAVG(I)=SAVG(I)/XMCN
                    SPLTDUM(1,M)=SNGL(SAVG(1))
                    SPLTDUM(2,M)=SNGL(SAVG(2))
                    SPLTDUM(3,M)=SNGL(SAVG(4))
                    SPLTDUM(4,M)=SNGL(SAVG(3))
730      CONTINUE
            WRITE(10) ((SPLTDUM(I,J),J=1,NE),I=1,4)
740      CONTINUE
750      CONTINUE
760      CONTINUE

        RETURN
        END

```

```

-----
-----
*
*      Subroutine SCRUBSV: This subroutine opens and closes the files
*      used by the main program, SCRUBS. The files:
*      UNITNAME
*      2SCRUBS.RST
*      3scratch file
*      4scratch file
*      5SCRUBS.DAT
*      6SCRUBS.LIS
*      8scratch file
*      9QMESH9.DAT
*      10SCRUBS.MOV
*      14RUB.MOV
*
*      Written:
*      Last modified:MAR 19 1984
*
-----
-----

```

```

*      Subroutines called:SCRUBSEXIT
*
-----
-----

```

```

PROGRAM SCRUBSV

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3  TPWORK,KNE
COMMON/QMESH/NEL

```

```

-----
*
*      The files are opened.
*
-----

```

```

OPEN(UNIT=2,TYPE='UNKNOWN',ACCESS='SEQUENTIAL',
1  NAME='SCRUBS.RST',FORM='UNFORMATTED')
OPEN(UNIT=3,TYPE='SCRATCH',ACCESS='SEQUENTIAL',

```

```

1      FORM='UNFORMATTED')
      OPEN(UNIT=4,TYPE='SCRATCH',ACCESS='SEQUENTIAL',
1      FORM='UNFORMATTED')
      OPEN(UNIT=5,TYPE='OLD',ACCESS='SEQUENTIAL',NAME='SCRUBS.DAT')
      OPEN(UNIT=6,TYPE='NEW',ACCESS='SEQUENTIAL',NAME='SCRUBS.LIS')
      OPEN(UNIT=10,TYPE='NEW',ACCESS='SEQUENTIAL',NAME='SCRUBS.MOV',
1      FORM='UNFORMATTED')
      OPEN(UNIT=9,TYPE='UNKNOWN',ACCESS='SEQUENTIAL',
1      NAME='QMESH9.DAT',FORM='UNFORMATTED')
      OPEN(UNIT=8,TYPE='SCRATCH',ACCESS='SEQUENTIAL',
1      FORM='UNFORMATTED')

```

•

• The main program, SCRUBS, is called.

•

```

      CALL SCRUBS

```

•

• The files are closed, and the system call EXIT is used.

•

```

      CLOSE(UNIT=2)
      CLOSE(UNIT=3)
      CLOSE(UNIT=4)
      CLOSE(UNIT=5)
      CLOSE(UNIT=6)
      IF(NEL EQ 0) CLOSE(UNIT=9)
      IF(NEL NE 0) CLOSE(UNIT=9,DISPOSE='DELETE')
      IF(NL EQ 1)CLOSE(UNIT=10)
      CLOSE(UNIT=8)
      CALL EXIT

```

```

      STOP

```

```

END

```

```

-----
-----
•
• Subroutine SET:
• Sets element spacing and determines bandwidth and resulting
• stiffness storage required.
•

```

```

• Written:
• Last modified: MAR 19 1984
•

```

```

-----
-----
•
• Called by: SCRUBS
•

```

```

-----
-----
SUBROUTINE SET

```

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS
1 ,NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2 NSTORE
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1 ,NOP(4000),IMAT(1000),ION(6,1000)

DO 10 J=1,6
  DO 10 I=1,NP
10 ION(J,I)=0

```

```

-----
-----
•
• Selects spacing.
•

```

```

DO 50 L=1,NE
  NASH=NCN*(L-1)
  IST=NOP(NASH+1)
  MAX=IST
  MIN=IST
  DO 20 N=2,NCN
    NTH=NOP(NASH+N)

```

```

      IF (NTH .EQ. 0) GO TO 30
      IF ( MAX .LT. NTH) MAX=NTH
      IF ( MIN .LE. NTH) GO TO 20
      MIN=NTH
20    CONTINUE
30    IF (NSIZ .LT. (MAX-MIN)*NDF) NSIZ=(MAX-MIN)*NDF
      I=MIN
      DO 40 M=1,6
        IF(ION(M,I).NE.0) GO TO 40
        ION(M,I)=L
        GO TO 50
40    CONTINUE
50    CONTINUE
      L=1
      DO 70 I=1,NP
        DO 60 M=1,6
          IF(ION(M,I).EQ.0) GO TO 70
          IONARY(L)=ION(M,I)
60    L=L+1
70    CONTINUE
      IONARY(L)=0

```

•
 •
 • Determines if bandwidth is too high and stiffness dimensioning
 • is exceeded.
 •
 •

```

      NSK=((NSIZ+NDF)*(NSIZ+NDF+1))/2+1
      IF(NSK.LT.NSZF+1) NSK=NSZF+1
      IF(NSK.LE.9000) RETURN
      WRITE(6,80)
80    FORMAT(' PROGRAM HALTED IN SET, '
1     'STIFFNESS SPACE EXCEEDED')

```

```

      STOP
      END

```

•
 • Subroutine SFR:
 • Sets shape functions according to element type.

•
 • Written:
 • Last modified: JUL 14 1984

•
 • Called by: LDATAREMLDRESIDUE
 • STIFM

SUBROUTINE SFR(G,H)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
 COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
 1 NCORD
 COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),
 1 NOPE(12),DISE(2,12),RP,RPB,XYP(3)

•
 • G and H denote the xi and eta values at the point considered.
 •

GG = G*G
 GH = G*H
 HH = H*H
 GGH = GG*H
 GHH = G*HH
 IF(NSFR-2)10,20,30

•
 • Linear shape functions and their first derivatives for
 • four noded arbitrary quadrilateral.
 •

```

10      P(1)=(1.-G-H+GH)/4.
        P(2)=(1.+G-H-GH)/4.
        P(3)=(1.+G+H+GH)/4.
        P(4)=(1.-G+H-GH)/4.
        DEL(1,1)=(-1.+H)/4.
        DEL(1,2)=-DEL(1,1)
        DEL(1,3)=(1.+H)/4.
        DEL(1,4)=-DEL(1,3)
        DEL(2,1)=(-1.+G)/4.
        DEL(2,2)=(-1.-G)/4.
        DEL(2,3)=-DEL(2,2)
        DEL(2,4)=-DEL(2,1)
        RETURN

```

•
•
• Parabolic shape functions and their first derivatives for
• curved 8-nodal arbitrary quadrilateral.
•
•

```

20      G2 = G*2.
        H2 = H*2.
        GH2 = GH*2.
        P( 1) = (-1.+GH+GG+HH-GGH-GHH)/4.
        P( 2) = ( 1.-H-GG+GGH)/2.
        P( 3) = (-1.-GH+GG+HH-GGH+GHH)/4.
        P( 4) = ( 1.+G-HH-GHH)/2.
        P( 5) = (-1.+GH+GG+HH+GGH+GHH)/4.
        P( 6) = ( 1.+H-GG-GGH)/2.
        P( 7) = (-1.-GH+GG+HH+GGH-GHH)/4.
        P( 8) = ( 1.-G-HH+GHH)/2.
        DEL(1,1)=( H+G2-GH2-HH)/4.
        DEL(1,2)= -G+GH
        DEL(1,3)=(-H+G2-GH2+HH)/4.
        DEL(1,4)=( 1.-HH)/2.
        DEL(1,5)=( H+G2+GH2+HH)/4.
        DEL(1,6)= -G-GH
        DEL(1,7)=(-H+G2+GH2-HH)/4.
        DEL(1,8)=(-1.+HH)/2.
        DEL(2,1)=( G+H2-GG-GH2)/4.
        DEL(2,2)=(-1.+GG)/2.
        DEL(2,3)=(-G+H2-GG+GH2)/4.
        DEL(2,4)= -H-GH
        DEL(2,5)=( G+H2+GG+GH2)/4.

```

```

DEL(2,6)=( 1.-GG)/2.
DEL(2,7)=(-G+H2+GG-GH2)/4.
DEL(2,8)= -H+GH
RETURN
30 CONTINUE
GGG = GG•G
HHH = H•HH
GGGH = GGG•H
GHHH = G•HHH

```

•
•
• Cubic shape functions and their first derivatives for
• curved 12-nodal arbitrary quadrilateral.
•

```

G9 = G•9.
G10 = G•10.
G18 = G•18.
G27 = G•27.
H9 = H•9.
H10 = H•10.
H18 = H•18.
H27 = H•27.
GG9 = GG•9.
GG27 = GG•27.
GG81 = GG•81.
GH10 = GH•10.
GH18 = GH•18.
GH27 = GH•27.
HH9 = HH•9.
HH27 = HH•27.
HH81 = HH•81.
GGG9 = GGG•9.
GGG27 = GGG•27.
GGH9 = GGH•9.
GGH27 = GGH•27.
GGH81 = GGH•81.
GHH9 = GHH•9.
GHH27 = GHH•27.
GHH81 = GHH•81.
HHH9 = HHH•9.
HHH27 = HHH•27.
GGGH9 = GGGH•9.

```

GGGH27 = GGGH*27.
GHHH9 = GHHH*9.
GHHH27 = GHHH*27.
P(1)= -10.+G10+H10+GG9-GH10+HH9-GGG9-GGH9-GHH9-HHH9+GGGH9+GHHH9
P(2)= 9.-G27-H9-GG9+GH27+GGG27+GGH9-GGGH27
P(3)= 9.+G27-H9-GG9-GH27-GGG27+GGH9+GGGH27
P(4)= -10.-G10+H10+GG9+GH10+HH9+GGG9-GGH9+GHH9-HHH9-GGGH9-GHHH9
P(5)= 9.+G9-H27-GH27-HH9-GHH9+HHH27+GHHH27
P(6)= 9.+G9+H27+GH27-HH9-GHH9-HHH27-GHHH27
P(7)= -10.-G10-H10+GG9-GH10+HH9+GGG9+GGH9+GHH9+HHH9+GGGH9+GHHH9
P(8)= 9.+G27+H9-GG9+GH27-GGG27-GGH9-GGGH27
P(9)= 9.-G27+H9-GG9-GH27+GGG27-GGH9+GGGH27
P(10)=-10.+G10-H10+GG9+GH10+HH9-GGG9+GGH9-GHH9+HHH9-GGGH9-GHHH9
P(11)= 9.-G9+H27-GH27-HH9+GHH9-HHH27+GHHH27
P(12)= 9.-G9-H27+GH27-HH9+GHH9+HHH27-GHHH27
DEL(1, 1)= 10.+G18-H10-GG27-GH18-HH9+GGH27+HHH9
DEL(1, 2)=-27.-G18+H27+GG81+GH18-GGH81
DEL(1, 3)= 27.-G18-H27-GG81+GH18+GGH81
DEL(1, 4)=-10.+G18+H10+GG27-GH18+HH9-GGH27-HHH9
DEL(1, 5)= 9.-H27-HH9+HHH27
DEL(1, 6)= 9.+H27-HH9-HHH27
DEL(1, 7)=-10.+G18-H10+GG27+GH18+HH9+GGH27+HHH9
DEL(1, 8)= 27.-G18+H27-GG81-GH18-GGH81
DEL(1,10)= 10.+G18+H10-GG27+GH18-HH9-GGH27-HHH9
DEL(1, 9)=-27.-G18-H27+GG81-GH18+GGH81
DEL(1,11)=-9.-H27+HH9+HHH27
DEL(1,12)=-9.+H27+HH9-HHH27
DEL(2, 1)= 10.-G10+H18-GG9-GH18-HH27+GGG9+GHH27
DEL(2, 2)=-9.+G27+GG9-GGG27
DEL(2, 3)=-9.-G27+GG9+GGG27
DEL(2, 4)= 10.+G10+H18-GG9+GH18-HH27-GGG9-GHH27
DEL(2, 5)=-27.-G27-H18-GH18+HH81+GHH81
DEL(2, 6)= 27.+G27-H18-GH18-HH81-GHH81
DEL(2, 7)=-10.-G10+H18+GG9+GH18+HH27+GGG9+GHH27
DEL(2, 8)= 9.+G27-GG9-GGG27
DEL(2, 9)= 9.-G27-GG9+GGG27
DEL(2,10)=-10.+G10+H18+GG9-GH18+HH27-GGG9-GHH27
DEL(2,11)= 27.-G27-H18+GH18-HH81+GHH81
DEL(2,12)=-27.+G27-H18+GH18+HH81-GHH81
DO 50 J=1,12
DO 40 I=1,2
DEL(I,J)=DEL(I,J)/32.
CONTINUE
P(J)=P(J)/32.
CONTINUE

RETURN
END

```

-----
-----
*
*       Subroutine SLIPP:
*       This subroutine reads the slip plane data, adjusts the geometry
*       and elements accordingly, and then records the adjusted
*       information.
*
*       Written:
*       Last modified: MAR 19 1984
*
-----
-----

```

```

-----
-----
*
*       Called by: GDATA
*
-----
-----

```

```

-----
-----
*
*       Subroutines called: NEWNODCORNN
*       MAKEELNODEXY
*
-----
-----

```

SUBROUTINE SLIPP

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,

1 NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,

2 NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

3 TPWORK,KNE

COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),

1 US(400,2)

COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)

1 ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)

COMMON/SLP/DCOR(1000,2)

DIMENSION MAP(1000,2)

INTEGER SLIP(500,2),CORNER(500),EL

NSP=IABS(NNP)

```

-----
•
•
•   Saves old information.
•
•
-----

```

```

      NEOLD=NE
      NPOLD=NP
      DO 10 N=1, NP
        DCOR(N,1)=CORD(N,1)
10     DCOR(N,2)=CORD(N,2)

      WRITE(6,20)
20     FORMAT(///,1X,10('•'),' SLIP PANE BOUNDARY DATA ',10('•'))
      WRITE(6,30) NSP
30     FORMAT(//,' NUMBER OF SLIP PLANES = ',I2)

```

```

-----
•
•
•   Loops on number of slip planes.
•
•
-----

```

```

      DO 340 INSP=1, NSP

```

```

-----
•
•
•   Reads in slip plane data.
•
•
-----

```

```

      READ(5,40) NSLIP, IANG, DELTA
40     FORMAT(2I5, G10.3)

      K=(NSLIP-1)/16 + 1
      J=1
      DO 60 I=1, K
        JJ=J+15
        IF(JJ.GE.NSLIP) JJ=NSLIP
        READ(5,50) (SLIP(N,1), N=J, JJ)
50         FORMAT(16I5)
60         J=J+16
          IF(DELTA.LE.0) DELTA=(DABS(CORD(1,1)-CORD(2,1))+
+           DABS(CORD(1,2)-CORD(2,2)))/50.

```

•
•
• Changes slip node array to new node numbers.
•

```

      IF(INSP.LE.1) GOTO 80
      DO 70 N=1,NSLIP
70    CALL NEWNOD(SLIP(N,1))
80    CONTINUE
      L=4*NSFR
      NECN=NCN*NE

```

•
•
• Loops on number of slip plane nodes.
•

```

      DO 210 I=1,NSLIP
      NADD=1
      IX=SLIP(I,1)
      SLIP(I,2)=0
      CORDN=CORD(IX,IANG)+DELTA

```

•
•
• Determines if node is a corner node.
•

```

      CALL CORNN(NADD,NSFR,NP,SLIP(I,2),IX)

```

•
•
• Adds NADD to all numbers in connectivity greater than IX.
•

```

      DO 120 J=1,NECN
      IXX=NOP(J)-IX
      IF(IXX)120,90,110
90    EL=(J+L-1)/L
      X=0.0
      MMM=L*(EL-1)

```

```

DO 100 M=1,L,NSFR
  MM=MMM+M
100  X=X+CORD(NOP(MM),IANG)/4.
      IF(X.LT.CORDN)GOTO 120
110  NOP(J)=NOP(J)+NADD
120  CONTINUE

```

•

• Updates remaining slip plane node numbers.

•

```

DO 130 N=1,NSLIP
130  IF(SLIP(N,1).GT.SLIP(1,1))SLIP(N,1)=SLIP(N,1)+NADD

```

•

• Updates coordinate array numbers (add NADD to those < IX).

•

```

DO 140 J=NP,IX,-1
  IJ=J+NADD
  CORD(IJ,1)=CORD(J,1)
140  CORD(IJ,2)=CORD(J,2)

```

•

• Assigns coordinate to new node created.

•

```

CORD(IJ,IANG)=CORDN
NP=NP+NADD
NUMDUM=0
JJ=NB

```

•

• Updates boundary arrays, giving new nodes same boundary conditions
as corresponding slip plane nodes.

•

```

      DO 200 JJJ=1, JJ
      J=JJJ+NUMDUM
      IXC=NBC(J)-IX
      IF(IXC)200,150,190
150   DO 180 II=1,NADD
      NB=NB+1
      IJ=J+II
      DO 170 N=NB,IJ,-1
      NBC(N)=NBC(N-1)
      NFIX(N)=NFIX(N-1)
      DO 160 NN=1,NDF
160   US(N,NN)=US((N-1),NN)
170   ANG(N)=ANG(N-1)
180   NBC(IJ)=NBC(IJ)+1
      NUMDUM=NADD
      GOTO 200
190   NBC(J)=NBC(J)+NADD
200   CONTINUE
210   CONTINUE

```

•

• Adds row of slip elements to connectivity.

•

• Sets up array of corner nodes.

•

```

      NCORN=0
      DO 220 I=1,NSLIP
      IF(SLIP(I,2).EQ 0)GOTO 220
      NCORN=NCORN+1
      CORNER(NCORN)=SLIP(I,1)
220   CONTINUE
      IQUIT=1
      K=0
      I=1

```

•

• Finds corner node in connectivity array.

•

```

230   J=0

```

```

240      J=J+1
      IF(J.GT.NECN)GOTO 290
      IF(NOP(J).NE.CORNER(1))GOTO 240

```

```

*
*      Determines element number of element containing node.
*

```

```

      EL=(J+L-1)/L

```

```

*
*      Finds other corner node contained in same element.
*

```

```

      MMM=L*(EL-1)
      DO 260 M=1,L,NSFR
        MM=MMM+M
      DO 250 N=1,NCORN
        IF(N.EQ.1)GOTO 250
        IF(NOP(MM).EQ.CORNER(N))K=N

```

```

250      CONTINUE

```

```

260      CONTINUE

```

```

*
*      Prints error if no such node exists.
*

```

```

      IF(K.GT.0)GOTO 280
      WRITE(6,270)
270      FORMAT(' ERROR-- HALTED IN SLIPP, ERROR IN CONNECTIVITY')
      STOP

```

```

*
*      Makes new element.
*

```

```

280      IF(K.EQ.KK)GOTO 240

```

CALL MAKEEL(MMM,CORNER(I),CORNER(K),IANG)

•
• Checks to see how many elements are made.
•

```

      IQUIT=IQUIT+1
      IF(IQUIT.GE.NCORN)GOTO 300
      KK=I
      IF(1.EQ.1)KKK=K
      I=K
  
```

•
• Branches to next element.
•

```

      GOTO 230
290    I=1
      KK=KKK
      GOTO 230
  
```

•
• Writes slip plane data.
•

```

300    CONTINUE
      WRITE(6,310) INSP,DELTA,IANG
310    FORMAT(/,' SLIP PLANE ',I2,
+         /,' THICKNESS=',G10.3,
+         /,' ANGLE FLAG=',I2,
+         /,' NODE LIST: ')
      KNUM=(NSLIP-1)/16.+1
      KSTART=1
      DO 330 ITT=1,KNUM
        KEND=KSTART+15
        IF(KEND.GT.NSLIP)KEND=NSLIP
      WRITE(6,320) (SLIP(J,1),J=KSTART,KEND)
320    FORMAT(16I5)
  
```

```

330     KSTART=KSTART+16
340         CONTINUE

-----
*
*           Writes mapping of old to new nodes into scrubs.lis.
*
-----

        J=0
        DO 350 I=1,NPOLD
            N=I
            CALL NEWNOD(N)
            IF(N.EQ.1)GOTO 350
            J=J+1
            MAP(J,1)=I
            MAP(J,2)=N
350     CONTINUE
        WRITE(6,360)
360     FORMAT(/,' ***** NODE NUMBERS WERE CHANGED DUE TO SLIP PLANE'
+ ' ADDITION *****',//,' NODAL MAPPING OF NODES CHANGED:',//
+ ' ,4X,4('ORIGINAL NEW  '),/,5X,4('NODE#  NODE#  '))
        K=J/4
        KK=K*4-J
        K1=0
        IF(KK.LE.-1)K1=1
        K2=K1
        IF(KK.LE.-2)K2=K2+1
        K3=K2
        IF(KK.LE.-3)K3=K3+1
        DO 380 M=1,K
            WRITE(6,370) MAP(M,1),MAP(M,2),MAP(M+K+K1,1),MAP(M+K+K1,2),
+ MAP(M+2*K+K2,1),MAP(M+2*K+K2,2),MAP(M+3*K+K3,1),MAP(M+3*K+K3,2)
370     FORMAT(6X,4(13,6X,13,7X))
380     CONTINUE
            WRITE(6,370) (MAP(N,1),MAP(N,2),N=K+1,K3*(K+1),K+1)
            WRITE(6,390)
390     FORMAT(/,' SLIP PLANE ELEMENTS CREATED:',/,7X,'ELEMENT #'
+ ' ,6X,'CONNECTIVITY',/)
            ISTART=L*NEOLD+1
            DO 410 N=NEOLD+1,NE
                IEND=ISTART+L-1
                WRITE(6,400) N,(NOP(I),I=ISTART,IEND)
400     FORMAT(10X,13,4X,12I5)
410     ISTART=IEND+1

```

Numbers new mid-side nodes.

```

N=NEOLD+1
IF(NSFR.GT.1)CALL NODEXY(N)

RETURN
END

```

Subroutine SOLVE:
Solves stiffness equations, modifies loads.

Written:
Last modified: JUL 14 1984

Called by: SCRUBS

SUBROUTINE SOLVE

```

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO, PWORK,
3  TPWORK
COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1  US(400,2)
COMMON/LDS/R1(2000),RL(2000),RT(2000)
COMMON/SCR/SK(20000) ,R
1  ,ESTIFM(24,24),NN(12),SKS(1200),NOPS(24),ACT(48)

NBF=10000
NSIZ=0
NNP=0

```

```

NBUF=NSK-1
NZ=1
ND=1
L=0
IF (NT) 10,60,10
10  WRITE (6,20)
20  FORMAT(' ELASTIC STIFFNESSES READ IN')
   READ(5,30)NPSP
30  FORMAT(16I5)
   NACT=NDF*NPSP
   NSKS=NACT*(NACT+1)/2
   READ(5,30)(NOPS(I),I=1,NPSP)
   READ(5,40)(ACT(I),I=1,NACT)
40  FORMAT(4E20.13)
   READ(5,40)(SKS(I),I=1,NSKS)
   MASH=0
   DO 50 I=1,NPSP
     NASH=(NOPS(I)-1)*NDF
     DO 50 J=1,NDF
       MASH=MASH+1
       NASH=NASH+1
50  R1(NASH)=R1(NASH)+ACT(MASH)
60  CONTINUE
   DO 70 N=1,NBF
70  SK(N)=0.
   NC=1
   READ(3)
1  N,(NN(I),I=1,NCN),((ESTIFM(I,J),J=1,LV),I=1,LV)
80  L=L+1
   DO 140 M=1,6
     IF(N.EQ.0) GO TO 150
     DO 90 I=1,NCN
       IF (NN(I) .EQ. L) GO TO 100
90  CONTINUE
     GO TO 150
100 NC=NC+1
     DO 130 I=1,NCN
       IF(NN(I).EQ.0) GO TO 130
       II=(NN(I)-L)*NDF
       DO 120 J=1,NCN
         IF(NN(J).EQ.0) GO TO 120
         JJ=(NN(J)-L)*NDF
         DO 110 IL=1,NDF
           IA=II+IL
           IC=(I-1)*NDF+IL

```

```

        DO 110 JM=1,NDF
            JB=JJ+JM
            IF(IA.GT.JB) GO TO 110
            JD=(J-1)*NDF+JM
            IAB=((JB-1)*JB)/2+IA
            SK(IAB)=SK(IAB)+ESTIFM(IC,JD)
110      CONTINUE
120      CONTINUE
            NX=(NN(1)-L)*NDF
            IF(NSIZ.LT.NX) NSIZ=NX
130      CONTINUE
            READ(3)
            1  N,(NN(I),I=1,NCN),((ESTIFM(I,J),J=1,LV),I=1,LV)
140      CONTINUE

```

```

-----
*
*
*      Sets up load.
*
-----

```

```

150      CONTINUE
            IF (NT) 200,200,160
160      DO 170 I1=1,NPSP
            IF(L.EQ.NOPS(I1))GO TO 180
170      CONTINUE
            GO TO 200
180      IJ=(I1-1)*NDF
            DO 190 I2=1,NDF
                II=IJ+I2
                DO 190 JJ=II,NACT
                    I3=(10*JJ/NDF+9)/10
                    I5=JJ*(JJ-1)/2+II
                    JB=NDF*(NOPS(I3)-L-I3+1)+JJ
                    IA=JB*(JB-1)/2+I2
                    SK(IA)=SK(IA)+SKS(I5)
190      CONTINUE
200      CONTINUE
            NSZ=NSIZ+NDF
            JZ=0
            IF(NZ.EQ.NB+1) GO TO 210
            IF(L.NE. NBC(NZ)) GO TO 210
            JZ=NFIX(NZ)
            IZ=10**(NDF-1)
            NZ=NZ+1

```

```

210      DO 390 I=1,NDF
          NSZ=NSZ-1
          IC=I+NDF*(L-1)
          R=R1(IC)
          NBD=0
          IF(JZ.EQ.0) GO TO 230
          IF(JZ.LT.IZ) GO TO 220
          LZ=(NZ-2)*NDF+I
          U(ND) =U(LZ)
          RS=-R
          R=U(ND)
          ND=ND+1
          NBD=1
          JZ=JZ-IZ
220      IZ=IZ/10
230      CONTINUE
          IF (SK(1) .GE. 0.) GO TO 250
          WRITE (6,240) L,I,SK(1)
240      FORMAT(' NODE NO ',I5,I10,' D.F. ',SK(1) =',E15.6)
          SK(1) = 0.
250      CONTINUE
          NBLK=NSZ+4
          IF((NBUF+NBLK).LE NBF) GO TO 260
          WRITE(4) NBUF,(SK(J),J=NSK,NBUF)
          NBUF=NSK-1
          NNP=0
260      NNP=NNP+1
          NBUF=NBUF+1
          SK(NBUF)=NSZ
          NBUF=NBUF+1
          IA=NBUF+1

```

```

.....
*
*      Invert diagonal term.
*
.....

```

```

          IF(NBD EQ 1) GO TO 300
          IF(SK(1).NE.0.) GO TO 280
          WRITE(6,270)L,I
270      FORMAT(' PROGRAM HALTED IN SOLVE'/
1         ' NEGATIVE OR ZERO DIAGONAL STIFFNESS'/
2         '     NODE NO.',I4,I10,' D.F. ')
          STOP

```



```

280      CONTINUE
        XK=1 /SK(1)
        SK(NBUF)=XK

-----
*
*      Modify loads and off-diagonal terms.
*
-----

290      R1(IC)=XK*R
        GO TO 310
300      SK(NBUF)=-SK(1)
        R1(IC)=SK(1)*R+RS
        XK=1.
        R=-R
310      CONTINUE
        IF(L+I-NP-NDF) 320,400,320
320      NBUF=NBUF+1
        KJ=1
        DO 330 J=1,NSZ
            KJ=KJ+J
            SK(NBUF)=XK*SK(KJ)
            IF(NBD.EQ.1) SK(NBUF)=-SK(NBUF)
330      NBUF=NBUF+1
        SK(NBUF)=NSZ
        NBUF=NBUF+1
        SK(NBUF)=NNP
        DO 360 J=1,NSZ
            IF(SK(IA).EQ.0.) GO TO 360
            IF(NBD.EQ.1) GO TO 350
            JB=(J*(J+1))/2+1
            KJ=1
            DO 340 K=1,J
                KJ=KJ+K
                IJ=JB+K
340      SK(IJ)=SK(IJ)-SK(KJ)*SK(IA)
350      JB=IC+J
            R1(JB)=R1(JB)-SK(IA)*R
360      IA=IA+1
        DO 370 J=1,NSZ
            IK=(J*(J-1))/2
            IJ=IK+J+1
            DO 370 K=1,J
                IA=IK+K

```

```

      JB=IJ+K
370     SK(IA)=SK(JB)
      IJ=(NSZ*(NSZ+1))/2+1
      IK=IJ+NSZ
      DO 380 K=IJ,IK
380     SK(K)=0.
390     CONTINUE
      NSIZ=NSIZ-NDF
      GO TO 80
400     WRITE(4) NBUF,(SK(J),J=NSK,NBUF)
      IF(NSIZ.EQ.0) NSIZ=NDF

      RETURN
      END

```

```

-----
-----
*
*
*       Subroutine STIFM:
*       Forms and updates stiffness matrix, and calculates body forces.
*
*       Written:
*       Last modified: JUL 14 1984
*
-----
-----

```

```

*
*       Called by: SCRUBS
*
-----
-----

```

```

*
*       Subroutines called: MODSFR
*       AUXINVARFLOW
*       ROSB
*
-----
-----

```

SUBROUTINE STIFM

IMPLICIT DOUBLE PRECISION (A-H,O-Z)

```

COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,

```

```

3      TPWORK,LNEW
      COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
      COMMON/BOUN/NBC(400),NFIX(400),U(800),ANG(400),TRC(800),
1      US(400,2)
      COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
      COMMON/MATP/D(4,4),YM,PR,GM,HM,PPR,CLAME,ALPHA,YST
1      ,TABSTN(10,15),TABSTR(10,15)
      COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),
1      NOPE(12),DISE(2,12),RP,RPB,XYP(3)
      COMMON/INV/SMEAN,ST J2,ST J3,SIGMA,PHI,APHI,STN,CPhi,YIELD
      COMMON/FLW/ABETA,AA(4),DD(4)
      COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1      ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
2      ,COJ(2,2),COJIN(2,2),B(4,24),ESTIFM(24,24),DVD(4,4),DVDB(4)
      COMMON/FG/ FLAG(1000),BULK(1000,2),MINES(10,50),NME(10),GAMMA
      DIMENSION STRS(4)

```

```

      NECN=NE*NCN

```

```

      READ (8)(NOP(J),J=1,NECN),(IMAT(I),I=1,NE),(((DISN(K,I),
1      TDIS(K,I)),K=1,2),(CORD(I,J),J=1,NCORD ),I=1,NP),(((TSTS(I,J),
2      EPSTN(I,J)),I=1,5),J=1,NSTORE)
      REWIND 8
      DO 170 M=1,NE
          LNEW=IONARY(M)
          NASH=NCN*(LNEW-1)
          NG=MCN*(LNEW-1)
          L=IMAT(LNEW)

```

```

      Uses new stiffness for failed material.

```

```

      IF(NL.NE.1) GO TO 10
      IF(BULK(LNEW,1).NE.0.) L=NMAT
10     CONTINUE

```

```

      Note material no. of element along sloping boundary is greater
      than NMAT by 100.

```

```

IF(L.GT.NMAT) L=L-100
DO 30 K=1,NCN
  MASH=NOP(NASH+K)
  DO 20 I=1,NCORD
    XYE(I,K)=CORD(MASH,I)
20  CONTINUE
  NOPE(K)=MASH
30  CONTINUE
  CALL MOD(L,0)

```

•

• Sets initial values as zeroes.

•

```

DO 40 I=1,LV
  DO 40 J=1,LV
    ESTIFM(I,J)=0.
    IF(I.LE.4) B(I,J)=0.
40  CONTINUE
  THICK=ORT(L,6)
  IF(THICK.EQ.0.)THICK=1.

```

•

• Formulates stiffness.

•

```

DO 160 IGAUS=1,NGAUS
  DO 160 JGAUS=1,NGAUS
    G=XG(IGAUS)
    H=XG(JGAUS)
    CALL SFR(G,H)
    CALL AUX(M)
    DV=DET*CG(IGAUS)*CG(JGAUS)*THICK
    IF(NPP.EQ.2)DV=DV*RP*6.283185307179586
    IF(NCORD.EQ.3)DV=DV*XYP(3)

```

•

• Calculates B matrix from DI matrix.

•

```

NASH=0
DO 50 I=1,NCN
  MASH=NASH+1
  NASH=MASH+1
  B(1,MASH)=DI(1,I)
  B(2,NASH)=DI(2,I)
  B(3,MASH)=DI(2,I)
  B(3,NASH)=DI(1,I)
  IF(NPP.EQ.2) B(4,MASH)=P(I)/RP
50  CONTINUE

```

•
•
•
•
•
•

Calculates DVD matrix.

```

DO 60 I=1,ICS
  DO 60 J=1,ICS
    DVD(I,J)=DV*D(I,J)
60  CONTINUE

```

•
•
•
•
•
•

Tangential stiffness calculations.

```

NG=NG+1
PSTY=TS(5,NG)
PSTRN=EPSTN(5,NG)
IF(ORT(L,2).LT.0.0) GO TO 110
IF(PSTRN.EQ.0.) GO TO 110
MASH=0
70  MASH=MASH+1
    IF(PSTRN.LT.TABSTN(L,MASH)) GO TO 80
    GO TO 70
80  NASH=MASH
    MASH=MASH-1
    ETAB=TABSTN(L,NASH)-TABSTN(L,MASH)
    GASH=(PSTRN-TABSTN(L,MASH))/ETAB
    CYIELD=TABSTR(L,MASH)*(1.-GASH)+GASH*TABSTR(L,NASH)
    IF(PSTY.LT..9999*CYIELD) GO TO 110
    HM=(TABSTR(L,NASH)-TABSTR(L,MASH))/ETAB
    DO 90 I=1,4

```

```

90      STRS(I)=TSTS(I,NG)
        CALL INVAR (STRS)
        CALL NFLOW(STRS)
        CON=DV*ABETA
        DO 100 I=1,ICS
            DO 100 J=1,ICS
100     DVD(I,J)=DV-CON*DD(I)*DD(J)
110     DO 150 J=1,LV
            DO 130 K=1,ICS
                GASH=0.
                DO 120 I=1,ICS
120         GASH=GASH+DVD(K,I)*B(I,J)
130         DVDB(K)=GASH
                DO 150 I=J,LV
                    GASH=ESTIFM(I,J)
                    DO 140 K=1,ICS
140         GASH=GASH+B(K,I)*DVDB(K)
                    ESTIFM(J,I)=GASH
150         ESTIFM(I,J)=GASH
160     CONTINUE
        IF(IMAT(LNEW)GT.NMAT) CALL ROSB
        WRITE(3)
1      LNEW,(NOPE(I),I=1,NCN),((ESTIFM(I,J),J=1,LV),I=1,LV)
170     CONTINUE
        A=0.
        N=0
        WRITE(3)N,(N,I=1,NCN),((A,J=1,LV),I=1,LV)
        REWIND 3

        RETURN
        END

```

```
.....  
.....  
.  
.  
Subroutine SWITCH:  
Switches coordinates.  
.  
.  
Written:  
Last modified: JUL 19 1984  
.  
.....  
.....
```

```
.  
.  
Called by: FTEST  
.  
.....  
.....
```

```
SUBROUTINE SWITCH(J,K)
```

```
I=K  
K=J  
J=I
```

```
RETURN  
END
```

```
.....  
.....  
.  
.  
Subroutine TFORM:  
Makes adjustments for inclined boundary situations.  
.  
.  
Written:  
Last modified: JUL 14 1984  
.  
.....  
.....
```

```
.  
.  
Called by: SCRUBS  
.  
.....  
.....
```

```
SUBROUTINE TFORM
```

```
IMPLICIT DOUBLE PRECISION (A-H,O-Z)
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1  NCORD
COMMON/BOUN/NBC(400),NFX(400),U(800),ANG(400),TRC(800),
1  US(400,2)
COMMON/LDS/R1(2000),RL(2000),RT(2000)

DO 10 M=1,NB
  IF(ANG(M) EQ 0.) GOTO 10
  N=NBC(M)
  NASH=2*N
  MASH=NASH-1
  GASH=ANG(M)*.017453292
  CS=COS(GASH)
  TN=TAN(GASH)
  RXC=R1(MASH)*CS
  RYC=R1(NASH)*CS
  R1(MASH)=RXC+RYC*TN
  R1(NASH)=RYC-RXC*TN
10 CONTINUE

RETURN
END
```



```

-----
-----
*
*
*   Subroutine AUX:
*   Creates coordinate jacobian 0 interpolate X, YOCOJ(I,J),XY(1),XY(2)
*   are respectively the coordinate jacobian matrix, the X
*   coordinate and the Y coordinate.
*
*
*   Written:
*   Last modified:MAR 16 1984
*
-----
-----

```

```

-----
-----
*
*   Called by: LDATAREMLDRESIDUE
*   STIFM
*
-----
-----

```

SUBROUTINE AUX(M)

```

      IMPLICIT DOUBLE PRECISION (A-H,O-Z)
      COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
1     NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
2     NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
3     TFWORK,LNEW
      COMMON/NUME/P(12),DEL(2,12),DET,DI(2,12),DIJ(2,2),XYE(3,12),
1     NOPE(12),DISE(2,12),RP,RPB,XYP(3)
      COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
1     ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000)
2     ,COJ(2,2),COJIN(2,2)

      DO 20 I = 1,2
      DO 20 J = 1,2
      GASH = 0.
      DO 10 K = 1,NCN
         GASH = GASH+XYE(I,K)*DEL(J,K)
10     CONTINUE
      COJ(I,J) = GASH
20     CONTINUE
      DO 40 I = 1,NCORD
      GASH = 0.
      DO 30 K = 1,NCN
         GASH = GASH+XYE(I,K)*P(K)

```

```

30      CONTINUE
        XYP(1) = GASH
40      CONTINUE
        RP = XYP(1)

-----
*
*
*      Calculates the determinant and inverse of the coordinate Jacobian.
*      DET and COJIN(I,J) are determinant and inverse coordinate
*      jacobian, respectively.
*
-----

        DET = COJ(1,1)*COJ(2,2)-COJ(1,2)*COJ(2,1)
        IF(DET)50,70,90
50      CONTINUE
C       WRITE(6,60)M,((XYE(JJ,II),JJ = 1,2),II = 1,4),DET,COJ(1,1),
C + COJ(2,2),COJ(1,2),COJ(2,1)
60      FORMAT(' NEGATIVE DET STOPPED AT ELEMENT NO =',
+           I3,/, ' X1,Y1',2F8.2,/, ' X2,Y2',2F8.2,/, ' X3,Y3',2F8.2,
+           /, ' X4,Y4',2F8.2,/, ' DET',D12.4,/, ' COJ(1,1),COJ(2,2)',
+           ,2D12.4,/, ' COJ(1,2),COJ(2,1)',2D12.4)
c       STOP
70      DET = .000001
        WRITE(6,80)M
80      FORMAT(' ZERO DETERMINENT EL NO ',I3)
90      CONTINUE
        COJIN(1,1) = COJ(2,2)/DET
        COJIN(2,2) = COJ(1,1)/DET
        COJIN(1,2) = -COJ(1,2)/DET
        COJIN(2,1) = -COJ(2,1)/DET

-----
*
*
*      Calculates cartesian derivatives of shape functions DI(I,K), I=X,Y.
*
-----

        DO 120 I = 1,2
          DO 110 K = 1,NCN
            GASH = 0.
            DO 100 J = 1,2
              GASH = GASH+DEL(J,K)*COJIN(J,I)
100          CONTINUE
            DI(I,K) = GASH

```

110 CONTINUE
 120 CONTINUE

RETURN
 END

```

-----
-----
*
*
*   Subroutine ZONE:
*   This subroutine reads meshing cards as done in the CHILES2
*   program. It is provided here only as an interim capability.
*   This subroutine is called by the main program.
*
*   Written:
*   Last modified: JUL 19 1984
*
-----
-----

```

```

-----
-----
*
*   Called by: SCRUBS
*
-----
-----

```

SUBROUTINE ZONE (NRS)

IMPLICIT DOUBLE PRECISION (A-H,O-Z)
 COMMON/SCR/ R(2000),Z(2000),IX(5,2000),CODE(2000)
 COMMON/QMESH/NEL

```

-----
*
*   Reads element properties from SCRUBS.DAT.
*
-----

```

```

10 READ(5,10) NEL,NODES,NUMPC
   FORMAT (6I5)
   IBOMB=0
   IF(NEL.NE.0) GO TO 30
   WRITE(6,20)
20  FORMAT(' *****MESH GENERATOR QMESH USED FOR GRID INPUT*****')
   RETURN
30  CONTINUE

```

```

WRITE(6,40) NEL,NODES,NUMPC
40  FORMAT (4X,'NUMBER OF ELEMENTS (NEL)-----'
1    '-----',15/4X,'NUMBER OF NODAL POINTS (NODES)----'
2    '-----',15/4X,'NUMBER OF PRESSURE CARDS '
3    '(NUMPC)-----',15)

```

```

-----
•
•
•   NEL = 0 for mesh generator, NEL = -1 for restart.
•
•
-----

```

```

IF (NEL.LT.0) NRS=1
IF(NRS.EQ.1) RETURN
NFORCE=NUMPC
NUMSC=NUMPC
N=0
50  READ(5,10) M,(IX(I,M),I=1,5)
60  N=N+1
IF (M-N) 100,80,70
70  IX(1,N)=IX(1,N-1)+1
IX(2,N)=IX(2,N-1)+1
IX(3,N)=IX(3,N-1)+1
IX(4,N)=IX(4,N-1)+1
IX(5,N)=IX(5,N-1)
80  IF (M-N) 100,90,60
90  IF (NEL-N) 120,120,50
100 WRITE(6,110) M
110 FORMAT (' ***FATAL ERROR  ELEMENT CARD, M=',15)
IBOMB=1
120 CONTINUE

```

```

-----
•
•
•   Read nodal point data.
•
•
-----

```

```

N=0
130 READ (5,140) M,CODE(M),R(M),Z(M)
140 FORMAT (15,F5.0,4E10.0)
NNL=N+1
IF (NNL.EQ.1) GO TO 150
ZX=FLOAT(M-N)
DR=(R(M)-R(N))/ZX

```

```

      DZ=(Z(M)-Z(N))/ZX
150    N=N+1
      IF (M-N) 180,170,160
160    IF (CODE(NNL-1).EQ.CODE(M)) CODE(N)=CODE(M)
      R(N)=R(N-1)+DR
      Z(N)=Z(N-1)+DZ
      GO TO 150
170    IF (NODES-M) 180,200,130
180    WRITE (6,190) M
190    FORMAT (' ***FATAL ERROR  NODAL POINT CARD, M=',15)
      IBOMB=1
200    CONTINUE
      WRITE(9) NEL,NODES
      WRITE(9) (R(I),Z(I),I=1,NODES)
      WRITE(9)((IX(I,M),I=1,5),M=1,NEL)
      REWIND 9

```

•

•

• Output mesh.

•

```

      MPRINT=0
      DO 230 I=1,NEL
          IF (MPRINT.NE.0) GO TO 210
          MPRINT=50
210        MPRINT=MPRINT-1
          WRITE(6,220) I,(IX(J,I),J=1,5)
220        FORMAT(11I3,4I6,I7)
230        CONTINUE
      MPRINT=0
      DO 260 I=1,NODES
          IF(MPRINT.NE.0) GO TO 240
          MPRINT=50
240        MPRINT=MPRINT-1
          WRITE(6,250) I,R(I),Z(I)
250        FORMAT(6X,I6,2F12.3)
260        CONTINUE

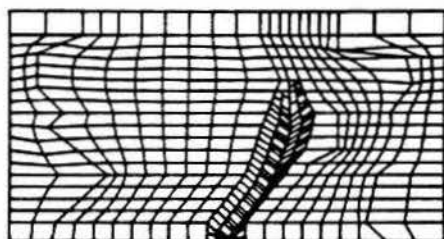
      RETURN
      END

```

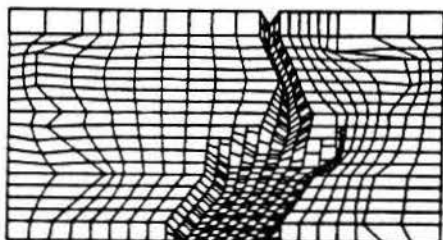
APPENDIX D

ADDITIONAL SUBSIDENCE RESULTS

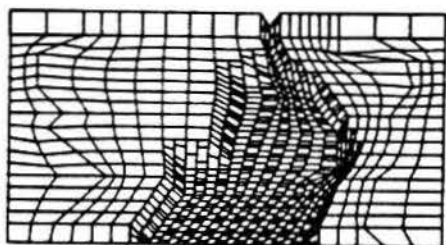
The following appendix contains additional results from running SCRUBS on the subsidence prediction problem found in chapter four. These three runs have been left out of the main report since they were results obtained during the course of trying to find good test parameters and were not considered to be as important to the conclusions as the tests included in the body of the report. However, it was thought that some readers may obtain valuable insights and information from them, so they have been included here. The test parameters varied in these tests were the failure stress. The failure stress used is listed in the figure heading.



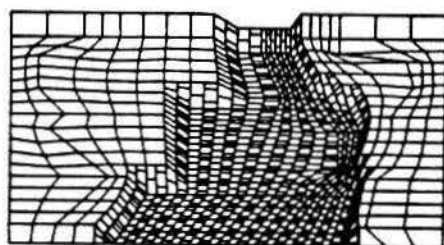
a. One support removed.



b. Three supports removed.

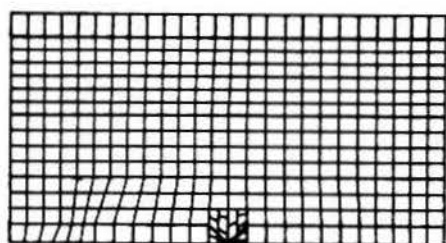


c. Five supports removed.

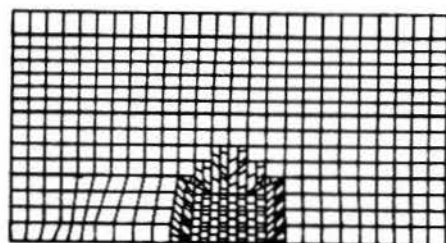


d. Seven supports removed.

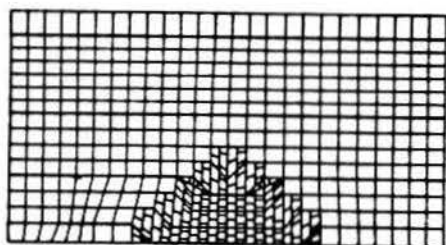
Figure 17 a-d. Jointed media skewed mesh. Failure stress was 0.1×10^5 dynes/cm².



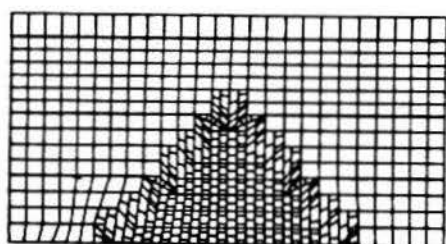
a. One support removed.



b. Three supports removed.

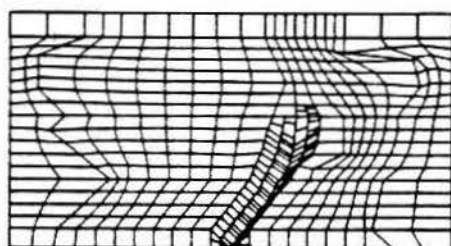


c. Five supports removed.

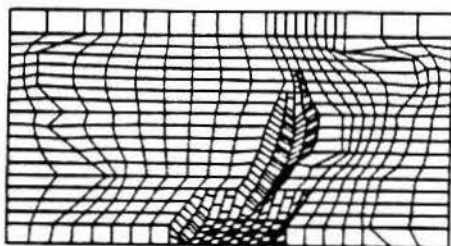


d. Seven supports removed.

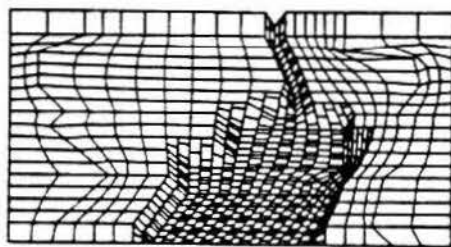
Figure 18 a-d. Discrete slip plane straight mesh. Failure stress was 0.1×10^9 dynes/cm².



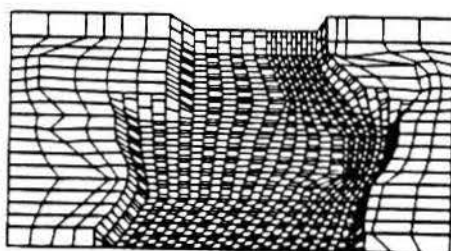
a. One support removed.



b. Three supports removed.



c. Five supports removed.



d. Seven supports removed.

Figure 19 a-d. Discrete slip plane skewed mesh. Failure stress was 0.1×10^5 dynes/cm².

APPENDIX E

SELECTED FILES

The following appendix contains typical files used in subsidence prediction with SCRUBS.BYU. First is the file FTEST, which is compiled with scrubs but is user-supplied (the one in this appendix is the one used to obtain our results), next are the two QMESH card input files used to create the meshes, and last is a typical SCRUBS.BYU data file used to obtain the combined skewed mesh model. To read the QMESH card files, the reader should consult reference 8. To read the SCRUBS.BYU data file, the user should consult appendix B, the SCRUBS.BYU user's manual. The SCRUBS.BYU subroutine FTEST is written in standard fortran and contains comments to show what is done.

CARD DESCRIPTIONS

The subroutine FTEST:

```

-----
-----
*
*      Subroutine FTEST:
*      This is a user supplied subroutine to determine the failure of
*      element M.
*
*      Written:
*      Last modified: JUL 19 1984
*
-----
-----

```

```

-----
-----
*
*      Called by: RESIDUE
*
-----
-----

```

```

-----
-----
*
*      Subroutines called:REMLDSWITCH
*
-----
-----

```

```
SUBROUTINE FTEST(M,SAVG,MATNO,IFLFG)
```

```
IMPLICIT DOUBLE PRECISION (A-H,O-Z)
```

```
COMMON/CONTR/TITLE(20),NP,NE,NB,NDF,NCN,NSFR,NSZF,LV,NPP,ICS,
```

```
1  NCORD,NSK,NSIZ,NL,NBUF,NT,ND,NNP,ILD,NLD,NMAT,IT,NIT,INC,NINC,
```

```
2  NSTORE,MPR,LDTYPE,FACTOR,OMEGA,CONFAC,NYIELD,ISTS,NALGO,PWORK,
```

```
3  TPWORK,KNE
```

```
COMMON/FG/ FLAG(1000),BULK(1000,2),MINE(10,50),NME(10)
```

```
COMMON/GEP/FAC(40),NOUT(40),ORT(10,10),NOPR(30),IONARY(1000)
```

```
DIMENSION SAVG(5),NM(4),MB(1000)
```

```
COMMON/SCR/DIS(2,1000),TDIS(2,1000),DISN(2,1000),CORD(1000,2)
```

```
1  ,NOP(4000),IMAT(1000),TSTS(5,4000),EPSTN(5,4000),DUMMY(1000)
```

```
COMMON/GID/XG(6),CG(6),XMG(6),CMG(6),NGAUS,MGAUS,MCN
```

```
IFLFG=0
```

```

-----
-----
*

```

- On first iteration, cycle 1, determines if element below
- element M has failed and creates MB(M) where MB(M)
- is the element number of the element below M.
-

```

IF (INC.GT.1) GO TO 70
IF (IT.GT.1) GO TO 70

```

-
- Finds four nodes associated with element M.
-

```

N1=NCN*(M-1)
DO 10 I=1,NCN,NSFR
  IX=I+N1
10  NM(I)=NOP(IX)

```

-
- Finds two lowest nodes of element M.
-

```

DO 30 I=1,3
  DO 20 I1=1,3
    I2=I1+1
20  IF(CORD(NM(I1),2).GT.CORD(NM(I2),2))CALL SWITCH(NM(I1),
& NM(I2))
30  CONTINUE

```

-
- Finds element below element M (this is the element which has the
- two bottom nodes of element M as its two top nodes).
-

```

MB(M)=-1
DO 60 I=1,NE
  IF(M.EQ.I) GO TO 60
  KKK=NCN*(I-1)
  IM=0

```

```

      DO 50 IG=1,NCN,NSFR
        KK=KKK+IG
        IF(NOP(KK) EQ NM(1).OR.NOP(KK).EQ.NM(2))GOTO 40
        GOTO 50
40      IM=IM+1
        IF(IM.EQ 2)MB(M)=1
50      CONTINUE
60      CONTINUE
70      CONTINUE

```

•
•
• For first iteration, checks for mined out or specified
• element failure.
•

```

      IF(IT.NE.1) GO TO 90
      NMEL=NME(INC)
      IF(NMEL.EQ 0) GO TO 90
      DO 80 I=1,NMEL
        IF(MINE(INC,I) EQ M) GO TO 170
80      CONTINUE

```

•
• Sets the failure criteria value to sigfail.
•

```

90      SIGFAIL=ORT(MATNO,7)

      IF(BULK(M,1).NE.0.0) RETURN

```

•
• If bottom element MB(M) has failed, and element (M) is a slide line,
• then fails the slide line directly.
•

```

      MBTEST=MB(M)
      IF (NNP.EQ 0) GO TO 110
      IF (MBTEST LT 0) GO TO 110
      IF(BULK(MBTEST,1).EQ.0) GO TO 110

```

IF (MATNO EQ NMAT-1) GO TO 170

110 CONTINUE

MSTRT=(M-1)*MCN
DO 160 I=1,MCN
INDEX=MSTRT+I

*

* Compares the failure criteria with the current state of
* stress at this point in the logic for a failure test.

*

* In this specific application, the failure measure is
* taken as either the vertical stress or the horizontal stress.

*

* Also, no element is allowed to fail if the element below
* it has not failed, nor if it has no elements below it.
* if the element is a slide line, fail with no stress check.

*

* TSTS(1,INDEX)=HORIZONTAL STRESS
* TSTS(2,INDEX)=VERTICAL STRESS

*

* BULK(M,1)=0 0ELEMENT HAS NOT FAILED
* BULK(M,1)=1 0ELEMENT HAS FAILED

*

STR1=TSTS(1,INDEX)

STR2=TSTS(2,INDEX)

IF(MBTEST LT 0) RETURN

& IF((STR1.GT.SIGFAIL.AND.BULK(MBTEST,1).NE.0.0).OR.
(STR2.GT.SIGFAIL.AND.BULK(MBTEST,1).NE.0.0)) GOTO 170

160 CONTINUE

RETURN

*

* States that element has failed, and increases value of flag
* by 1 on each node.

*

170 CONTINUE

```

IFLFG=1
WRITE(6,180) M
180  FORMAT(' * * * * * ELEMENT ',I5,' HAS BECOME RUBBLE '
&      ' * * * * * ')
      DO 190 I=1,4
          KK = 4*(M-1)+I
          NODE=NOP(KK)
190    FLAG(NODE)=FLAG(NODE)+1.0

```

•
•
• Sets BULK(M,1) = 1.0 for failed element.
•

BULK(M,1) = 1.0

•
• Removes gravity loading from failed element.
•

CALL REMLD(M)

•
• Computes element area just prior to failure.
•

```

K1=(M-1)*4+1
K2=K1+1
K3=K2+1
K4=K3+1
J1=NOP(K1)
J2=NOP(K2)
J3=NOP(K3)
J4=NOP(K4)
COE1=CORD(J2,1)-CORD(J4,1)
COE2=CORD(J3,2)-CORD(J1,2)
COE3=CORD(J3,1)-CORD(J1,1)
COE4=CORD(J4,2)-CORD(J2,2)
AREA= 5*(COE1*COE2+COE3*COE4)
AOLD=BULK(M,2)

```

```

                BULK(M,2)=AREA
200            WRITE(6,210) AOLD,AREA
210            FORMAT(' AOLD ANEW ',2E15.5)

                RETURN
                END

```

This is the QMESH input card file used to create the skewed mesh (again, for those readers not familiar with QMESH, see reference 8) :

```

COMMEN  SKEWED MESH
POINT   1/  1.6500  62.4500      0
POINT  38/ 21.2000  62.4500      0
POINT  45/ 30.8000  62.4500      0
POINT  39/ 68.9000  62.4500      0
POINT  40/ 90.0000  62.4500      0
POINT   2/ 118.3500 62.4500      0
POINT   3/  1.6500  56.1000      0
POINT   4/ 21.2000  56.1000      0
POINT  46/ 30.8000  56.1000      0
POINT   5/ 68.9000  56.1000      0
POINT   6/ 90.0000  56.1000      0
POINT   7/ 118.3500 56.1000      0
POINT   8/ 90.0000  52.1000      0
POINT   9/  9.0000  51.1000      0
POINT  10/ 103.5000  50.1000      0
POINT  11/  9.0000  42.1000      0
POINT  12/ 74.5000  44.1000      0
POINT  13/ 19.0000  43.1000      0
POINT  14/  1.6500  35.1000      0
POINT  15/ 16.0000  35.1000      0
POINT  16/ 63.5000  35.1000      0
POINT  17/ 71.0000  35.1000      0
POINT  18/ 90.0000  35.1000      0
POINT  19/ 104.0000 35.1000      0
POINT  20/ 118.3500 35.1000      0
POINT  21/ 12.0000  31.1000      0
POINT  22/ 90.0000  25.1000      0
POINT  23/ 82.0000  21.1000      0
POINT  24/  1.6500  18.1000      0
POINT  25/ 28.0000  18.1000      0
POINT  47/ 33.0000  18.1000      0
POINT  26/ 60.0000  18.1000      0
POINT  27/ 65.0000  18.1000      0

```


POINT	28/	81.0000	18.1000		0
POINT	29/	100.5000	18.1000		0
POINT	30/	118.3500	18.1000		0
POINT	31/	1.6500	5.1000		0
POINT	32/	14.1000	5.1000		0
POINT	41/	24.3000	5.1000		0
POINT	33/	54.9000	5.1000		0
POINT	42/	95.7000	5.1000		0
POINT	34/	97.5000	5.1000		0
POINT	35/	118.3500	5.1000		0
POINT	36/	1.6500	0.0000		0
POINT	43/	24.3000	0.0000		0
POINT	44/	95.7000	0.0000		0
POINT	37/	118.3500	0.0000		0
LINE STR	138/	1 38	0 2	1.0000	0
LINE STR	13/	1 3	0 1	1.0000	100
LINE STR	345/	38 45	0 2	1.0000	0
LINE STR	439/	45 39	0 6	1.0000	0
LINE STR	340/	39 40	0 8	1.0000	0
LINE STR	402/	40 2	0 3	1.0000	0
LINE STR	27/	2 7	0 1	1.0000	100
LINE STR	34/	3 4	0 2	1.0000	40
LINE STR	314/	3 14	0 7	1.0000	100
LINE STR	446/	4 46	0 2	1.0000	40
LINE STR	49/	4 9	0 2	1.0000	50
LINE STR	465/	46 5	0 6	1.0000	40
LINE STR	56/	5 6	0 8	1.0000	40
LINE STR	512/	5 12	0 4	1.0000	60
LINE STR	67/	6 7	0 3	1.0000	40
LINE STR	68/	6 8	0 1	1.0000	90
LINE STR	720/	7 20	0 7	1.0000	100
LINE STR	810/	8 10	0 1	1.0000	90
LINE STR	911/	9 11	0 3	1.0000	50
LINE STR	113/	10 13	0 2	1.0000	90
LINE STR	115/	11 15	0 2	1.0000	50
LINE STR	117/	12 17	0 3	1.0000	60
LINE STR	119/	13 19	0 3	1.0000	90
LINE STR	415/	14 15	0 2	1.0000	30
LINE STR	124/	14 24	0 5	1.0000	100
LINE STR	116/	15 16	0 7	1.0000	30
LINE STR	121/	15 21	0 1	1.0000	50
LINE STR	617/	16 17	0 1	1.0000	30
LINE STR	126/	16 26	0 5	1.0000	70
LINE STR	118/	17 18	0 3	1.0000	30
LINE STR	819/	18 19	0 5	1.0000	30

LINE STR 122/ 18 22 0 3 1.0000 110
 LINE STR 120/ 19 20 0 3 1.0000 30
 LINE STR 230/ 20 30 0 5 1.0000 100
 LINE STR 225/ 21 25 0 4 1.0000 50
 LINE STR 223/ 22 23 0 1 1.0000 110
 LINE STR 228/ 23 28 0 1 1.0000 110
 LINE STR 425/ 24 25 0 2 1.0000 20
 LINE STR 231/ 24 31 0 4 1.0000 100
 LINE STR 247/ 25 47 0 2 1.0000 20
 LINE STR 232/ 25 32 0 4 1.0000 50
 LINE STR 426/ 47 26 0 5 1.0000 20
 LINE STR 227/ 26 27 0 1 1.0000 20
 LINE STR 728/ 27 28 0 3 1.0000 20
 LINE STR 233/ 27 33 0 4 1.0000 80
 LINE STR 229/ 28 29 0 6 1.0000 20
 LINE STR 930/ 29 30 0 2 1.0000 20
 LINE STR 234/ 29 34 0 4 1.0000 120
 LINE STR 335/ 30 35 0 4 1.0000 100
 LINE STR 332/ 31 32 0 2 1.0000 10
 LINE STR 336/ 31 36 0 1 1.0000 100
 LINE STR 341/ 32 41 0 2 1.0000 10
 LINE STR 433/ 41 33 0 6 1.0000 10
 LINE STR 342/ 33 42 0 8 1.0000 10
 LINE STR 434/ 42 34 0 1 1.0000 10
 LINE STR 435/ 34 35 0 2 1.0000 10
 LINE STR 337/ 35 37 0 1 1.0000 100
 LINE STR 343/ 36 43 0 4 1.0000 200
 LINE STR 444/ 43 44 0 14 1.0000 200
 LINE STR 437/ 44 37 0 3 1.0000 200
 SIDE 12/ 138 345 439 340 402
 SIDE 37/ 34 446 465 56 67
 SIDE 45/ 446 465
 SIDE 415/ 49 911 115
 SIDE 517/ 512 117
 SIDE 619/ 68 810 113 119
 SIDE 117/ 116 617
 SIDE 118/ 617 118
 SIDE 119/ 118 819
 SIDE 120/ 819 120
 SIDE 125/ 121 225
 SIDE 128/ 122 223 228
 SIDE 226/ 247 426
 SIDE 227/ 247 426 227
 SIDE 228/ 227 728
 SIDE 229/ 728 229

```

SIDE      230/ 229 930
SIDE      333/ 341 433
SIDE      334/ 342 434
SIDE      335/ 332 341 433 342 434 435
SIDE      337/ 343 444 437
REGION    1  1/ 12 -27 37 -13
REGION    2  2/ 45 517 117 415
REGION    2  3/-116 -126 226 125
REGION    2  4/ 227 -233 333 -232
REGION    2  5/ -56 619 119 517
REGION    2  6/ 118 128 228 -126
REGION    2  7/ 229 -234 334 -233
REGION    3  8/ -34 415 -415 -314
REGION    3  9/-415 125 -425 -124
REGION    3 10/-425 -232 -332 -231
REGION    3 11/ -67 -720 -120 619
REGION    3 12/ 120 -230 230 128
REGION    3 13/-930 -335 -435 -234
REGION    4 14/ 335 -337 337 -336
SCHEME    1M
SCHEME    2MS2S
SCHEME    3MS2S5S
SCHEME    4MS
SCHEME    5MS
SCHEME    6MS2S5S
SCHEME    7MS
SCHEME    8MS
SCHEME    9MS2S
SCHEME    10MS
SCHEME    11MS
SCHEME    12MS
SCHEME    13MS
SCHEME    14MS
END        4
P-L-      36 343 43 444 44 437 37
END

```

This is the QMESH input card file used to create the straight mesh (again, for those readers not familiar with QMESH, see reference 8) :

```

COMMENT   STRAIGHT MESH
POINT     1/  1.6500  62.4500      0
POINT     8/ 15.0000  62.4500      0
POINT    20/ 70.0000  62.4500      0

```

POINT	9/	100.0000	62.4500	0			
POINT	2/	118.3500	62.4500	0			
POINT	3/	1.6500	56.1000	0			
POINT	4/	15.0000	56.1000	0			
POINT	5/	70.0000	56.1000	0			
POINT	6/	100.0000	56.1000	0			
POINT	7/	118.3500	56.1000	0			
POINT	10/	1.6500	35.1000	0			
POINT	11/	15.0000	35.1000	0			
POINT	12/	60.0000	35.1000	0			
POINT	13/	70.0000	35.1000	0			
POINT	14/	85.0000	35.1000	0			
POINT	15/	100.0000	35.1000	0			
POINT	16/	118.3500	35.1000	0			
POINT	17/	1.6500	18.1000	0			
POINT	18/	15.0000	18.1000	0			
POINT	19/	60.0000	18.1000	0			
POINT	21/	85.0000	18.1000	0			
POINT	22/	100.0000	18.1000	0			
POINT	23/	118.3500	18.1000	0			
POINT	24/	1.6500	5.1000	0			
POINT	25/	15.0000	5.1000	0			
POINT	26/	24.3000	5.1000	0			
POINT	27/	60.0000	5.1000	0			
POINT	28/	95.7000	5.1000	0			
POINT	29/	100.0000	5.1000	0			
POINT	30/	118.3500	5.1000	0			
POINT	31/	1.6500	0.0000	0			
POINT	32/	24.3000	0.0000	0			
POINT	33/	95.7000	0.0000	0			
POINT	34/	118.3500	0.0000	0			
LINE STR	18/	1	8	0	3	1.0000	0
LINE STR	820/	8	20	0	12	1.0000	0
LINE STR	209/	20	9	0	6	1.0000	0
LINE STR	92/	9	2	0	4	1.0000	0
LINE STR	13/	1	3	0	1	1.0000	100
LINE STR	27/	2	7	0	1	1.0000	100
LINE STR	34/	3	4	0	3	1.0000	40
LINE STR	310/	3	10	0	6	1.0000	100
LINE STR	45/	4	5	0	12	1.0000	40
LINE STR	411/	4	11	0	6	1.0000	50
LINE STR	56/	5	6	0	6	1.0000	40
LINE STR	513/	5	13	0	6	1.0000	60
LINE STR	67/	6	7	0	4	1.0000	40
LINE STR	615/	6	15	0	6	1.0000	90

LINE STR 716/ 7 16 0 6 1.0000 100
 LINE STR 111/ 10 11 0 3 1.0000 30
 LINE STR 117/ 10 17 0 4 1.0000 100
 LINE STR 112/ 11 12 0 10 1.0000 30
 LINE STR 113/ 12 13 0 2 1.0000 30
 LINE STR 119/ 12 19 0 4 1.0000 80
 LINE STR 114/ 13 14 0 3 1.0000 30
 LINE STR 115/ 14 15 0 3 1.0000 30
 LINE STR 121/ 14 21 0 4 1.0000 130
 LINE STR 116/ 15 16 0 4 1.0000 30
 LINE STR 123/ 16 23 0 4 1.0000 100
 LINE STR 118/ 11 18 0 4 1.0000 50
 LINE STR 718/ 17 18 0 3 1.0000 20
 LINE STR 124/ 17 24 0 3 1.0000 100
 LINE STR 819/ 18 19 0 10 1.0000 20
 LINE STR 125/ 18 25 0 3 1.0000 50
 LINE STR 921/ 19 21 0 5 1.0000 20
 LINE STR 127/ 19 27 0 3 1.0000 80
 LINE STR 222/ 21 22 0 3 1.0000 20
 LINE STR 223/ 22 23 0 4 1.0000 20
 LINE STR 229/ 22 29 0 3 1.0000 140
 LINE STR 230/ 23 30 0 3 1.0000 100
 LINE STR 225/ 24 25 0 3 1.0000 10
 LINE STR 226/ 25 26 0 3 1.0000 10
 LINE STR 227/ 26 27 0 7 1.0000 10
 LINE STR 231/ 24 31 0 1 1.0000 100
 LINE STR 228/ 27 28 0 7 1.0000 10
 LINE STR 829/ 28 29 0 1 1.0000 10
 LINE STR 930/ 29 30 0 4 1.0000 10
 LINE STR 334/ 30 34 0 1 1.0000 100
 LINE STR 332/ 31 32 0 6 1.0000 200
 LINE STR 333/ 32 33 0 14 1.0000 200
 LINE STR 433/ 33 34 0 5 1.0000 200
 SIDE 12/ 18 820 209 92
 SIDE 37/ 34 45 56 67
 SIDE 113/ 112 113
 SIDE 114/ 113 114
 SIDE 115/ 114 115
 SIDE 116/ 115 116
 SIDE 122/ 921 222
 SIDE 223/ 222 223
 SIDE 230/ 225 226 227 228 829 930
 SIDE 227/ 226 227
 SIDE 229/ 228 829
 SIDE 334/ 332 333 433

```

REGION 1 1/ 12 -27 37 -13
REGION 2 2/ -45 -513 113 -411
REGION 2 3/-112 -119 -819 -118
REGION 2 4/-819 -127 227 -125
REGION 2 5/ -56 -615 115 -513
REGION 2 6/ 114 -121 -921 -119
REGION 2 7/ 122 -229 229 -127
REGION 3 8/ -34 -411 -111 -310
REGION 3 9/-111 -118 -718 -117
REGION 3 10/-718 -125 -225 -124
REGION 3 11/ -67 -716 -116 -615
REGION 3 12/ 116 -123 223 -121
REGION 3 13/-223 -230 -930 -229
REGION 4 14/ 230 -334 334 -231
SCHEME 1M
SCHEME 2M
SCHEME 3M
SCHEME 4M
SCHEME 5M
SCHEME 6M
SCHEME 7M
SCHEME 8M
SCHEME 9M
SCHEME 10M
SCHEME 11M
SCHEME 12M
SCHEME 13M
SCHEME 14M
END 4
P-L- 31 332 32 333 33 433 34
END

```

This is the SCRUBS.BYU data file used to create the failure and subsidence of Figure 15 a-d:

```

0
1
JOINTED MEDIA WITH HORIZONTAL SLIP – SKEWED MESH
0 0 58 1 2 6 1 2 3 0 1 0 1 0 2 2
1- 6000E+1110.6000E+060.1000E+21 1.000 0.0000E+000.0000E+00
2- 5500E+12- 2700E+120.1000E+21 1.000 0.0000E+000.0000E+00
10.000 0.10000E+09
3- 5500E+12- 2700E+120.1000E+21 1.000 0.0000E+000.0000E+00
10.000 0.10000E+09

```

```

40 2000E+130.3000 0.1000E+21 1.000 0.0000E+000.0000E+00
5-3450E+120.3450E+120.1000E+32 1.000 0.0000E+000.0000E+00
6-6000E+080.6000E+060.1000E+21 1.000 0.0000E+000.0000E+00
10.1000E+070.9800
20.1000E+070.9800
30.1000E+070.9800
40.0000E+00 60.00
50.0000E+00 1.000
60.0000E+000.0000E+00
0.0000E+00
5
0 4 8 8 8

224 228 368 369
140 144 232 236 366 367 370 371
132 136 240 244 364 365 372 373
124 128 248 252 362 363 374 375
4 0 58 0
1 11 0.000E+00 0.000E+00 0.000E+00
2 1 0.000E+00 0.000E+00 0.000E+00
3 1 0.000E+00 0.000E+00 0.000E+00
4 1 0.000E+00 0.000E+00 0.000E+00
5 1 0.000E+00 0.000E+00 0.000E+00
6 1 0.000E+00 0.000E+00 0.000E+00
7 1 0.000E+00 0.000E+00 0.000E+00
8 1 0.000E+00 0.000E+00 0.000E+00
9 1 0.000E+00 0.000E+00 0.000E+00
10 1 0.000E+00 0.000E+00 0.000E+00
11 1 0.000E+00 0.000E+00 0.000E+00
12 1 0.000E+00 0.000E+00 0.000E+00
13 1 0.000E+00 0.000E+00 0.000E+00
14 1 0.000E+00 0.000E+00 0.000E+00
15 1 0.000E+00 0.000E+00 0.000E+00
16 1 0.000E+00 0.000E+00 0.000E+00
17 1 0.000E+00 0.000E+00 0.000E+00
18 1 0.000E+00 0.000E+00 0.000E+00
19 1 0.000E+00 0.000E+00 0.000E+00
20 1 0.000E+00 0.000E+00 0.000E+00
21 1 0.000E+00 0.000E+00 0.000E+00
22 11 0.000E+00 0.000E+00 0.000E+00
23 10 0.000E+00 0.000E+00 0.000E+00
44 10 0.000E+00 0.000E+00 0.000E+00
45 10 0.000E+00 0.000E+00 0.000E+00
66 10 0.000E+00 0.000E+00 0.000E+00
67 10 0.000E+00 0.000E+00 0.000E+00

```

88 10 0.000E+00 0.000E+00 0.000E+00
 89 10 0.000E+00 0.000E+00 0.000E+00
 110 10 0.000E+00 0.000E+00 0.000E+00
 111 10 0.000E+00 0.000E+00 0.000E+00
 132 10 0.000E+00 0.000E+00 0.000E+00
 133 10 0.000E+00 0.000E+00 0.000E+00
 154 10 0.000E+00 0.000E+00 0.000E+00
 155 10 0.000E+00 0.000E+00 0.000E+00
 176 10 0.000E+00 0.000E+00 0.000E+00
 177 10 0.000E+00 0.000E+00 0.000E+00
 198 10 0.000E+00 0.000E+00 0.000E+00
 199 10 0.000E+00 0.000E+00 0.000E+00
 220 10 0.000E+00 0.000E+00 0.000E+00
 221 10 0.000E+00 0.000E+00 0.000E+00
 242 10 0.000E+00 0.000E+00 0.000E+00
 243 10 0.000E+00 0.000E+00 0.000E+00
 264 10 0.000E+00 0.000E+00 0.000E+00
 265 10 0.000E+00 0.000E+00 0.000E+00
 286 10 0.000E+00 0.000E+00 0.000E+00
 287 10 0.000E+00 0.000E+00 0.000E+00
 308 10 0.000E+00 0.000E+00 0.000E+00
 309 10 0.000E+00 0.000E+00 0.000E+00
 330 10 0.000E+00 0.000E+00 0.000E+00
 331 10 0.000E+00 0.000E+00 0.000E+00
 352 10 0.000E+00 0.000E+00 0.000E+00
 353 10 0.000E+00 0.000E+00 0.000E+00
 374 10 0.000E+00 0.000E+00 0.000E+00
 375 10 0.000E+00 0.000E+00 0.000E+00
 396 10 0.000E+00 0.000E+00 0.000E+00
 397 10 0.000E+00 0.000E+00 0.000E+00
 418 10 0.000E+00 0.000E+00 0.000E+00
 22 2 0.100
 27 28 29 30 31 32 33 25 26 41 42 34 35 36 37 38
 39 40 23 24 43 44
 22 2 0.100
 115 116 117 118 119 120 113 114 121 122 123 124 125 126 127 128
 129 130 111 112 131 132
 22 2 0.100
 230 231 223 224 225 226 227 228 229 234 235 236 237 238 239 232
 233 221 222 240 241 242
 22 2 0.100
 393 394 395 396 385 386 387 388 389 390 391 392 379 380 381 382
 383 384 377 378 375 376
 0 1

GRAVITY LOAD


```
0 0 1 0 0 0
0.0000E+00.14715E+06.00000E+00
1.2500 2.6400 1.6500 7.8000 3.3350 2.6400
5
1.0000.0000.0000.0000.0000
0 0 0 0 0
352.5000
0
0 1
```

APPLICATION OF COMBINED JOINTED MEDIA AND
DISCRETE SLIP PLANE CHARACTERISTICS
TO SUBSIDENCE PREDICTIONS

David W. Basinger

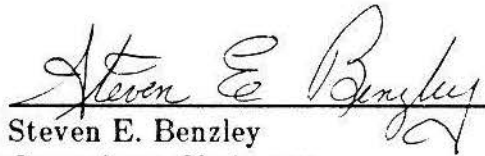
Department of Civil Engineering

M.S. Degree, December 1984

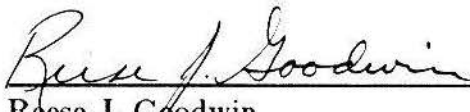
ABSTRACT

This thesis presents an application of a numerical formulation incorporating the effects of joints, cracks, and fractures to a soil subsidence prediction problem, and the extension of that formulation to combined discrete slip planes and jointed media continua formulations. The results obtained are compared to each other and to a physical centrifuge simulation performed previously on the same problem.

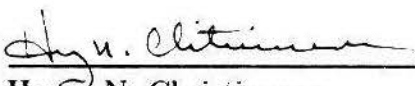
COMMITTEE APPROVAL:



Steven E. Benzley
Committee Chairman



Reese J. Goodwin
Committee Member



Henry N. Christiansen
Graduate Coordinator

